



MICROCOPY RESOLUTION TEST CHART





FOUNDATION ANALYSIS
EAST COAST AIR COMBAT MANEUVERING RANGE
OFFSHORE KITTY HAWK, NORTH CAROLINA
CONTRACT NO. N62477-76-C-0179
MODIFICATION NO. P0001

Report No. 27-771-97

Prepared for

NAVAL FACILITIES ENGINEERING COMMAND DEPARTMENT OF THE NAVY CHESAPEAKE DIVISION

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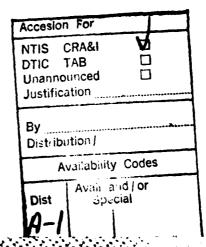
CREST ENGINEERING, INC. TULSA, OKLAHOMA

September 1976

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water depths (MLW) of 81 feet (one structure), 93 feet (one structure) and 105 feet (two structures). The structures are to be anchored to the seabed by driving 42" diameter piling through each leg of the templets and penetrating approximately 250 feet into the subsea soils. Vulcan 560 hammer or equivalent will be employed to drive the piling into the desired penetration.

SECTION 1

INTRODUCTION

1.1 INTRODUCTION

The objective of this report is to establish the design criteria for 42" diameter piling foundations to support four tripod-type ocean structures for the U.S. Navy Air Combat Maneuvering Range (ACMR) Offshore Kitty Hawk, North Carolina, U.S.A.

The four tripod-type structures are designed, respectively, for the water depths (MLW) of 81 feet (one structure), 93 feet (one structure) and 105 feet (two structures). The structures are to be anchored to the seabed by driving 42" diameter piling through each leg of the templets and penetrating approximately 250 feet into the subsea soils. Vulcan 560 hammer or equivalent will be employed to drive the piling into the desired penetration.

1.2 METHODS OF ANALYSIS

The method employed to perform the computation of pipe pile capacity curves, as presented in Section 2, is empirical in nature.

McClelland soil report (Volume No. 1, Foundation Investigation, Report to Cubic Corporation) serves as the basis of engineering data to develop the capacity curves for each boring site.

Stress-wave equation analysis is used in Section 3 to produce pile driving resistance curves. It should be noted that these curves provide only a possible range of pile drivability for the engineer to have a better judgment in the design of piling foundations and in no way assume attainment of the desired penetration.

1.3 PERSONNEL RESUMES

The personnel whose resumes follow were actively engaged on this project.



Chingmiin (Charlie) Chern

Senior Engineer

University	Degree	Year
National Taiwan	Bachelor of Science	
University	Civil Engineering	1961
North Dakota	Master of Science	
State University	Civil Engineering	1966
Lehigh	Ph.D.	
University	Civil Engineering	1969
Tulsa University	Graduate Study in	
•	Business Administration-	
	Management	1974

Societies, Licenses, and Other Activities:

Member American Society of Civil Engineers

Member International Association of Structural and
Bridge Engineers

Member American Society of Engineering Education

Member American Society of Engineering Education Registered Professional Engineer in Oklahoma

Experience:

1973 to Present

Senior Civil Engineer

Crest Offshore, Inc.

Engaged in the feasibility studies, structural analysis and design of offshore structures, equipment supports and other various types of petroleum related civil engineering works. Assignments include:

- ... Evaluation of engineering designs from other agencies.
- ... Analysis and design of offshore structures for oil industry.
- ... Analysis and design of supports and foundations for onshore refinery facilities.
- ... Development of a sequence of computer programs for the analysis of offshore structures.

Chingmiin (Charlie) Chern

Senior Civil Engineer

Experience Continued:

Engineer

Crew Chief

1969 to 1973 North Dakota State University

Associate Engaged in full-time lecture instruction for civil Professor of engineering (graduate school division) and construction management. Also served as consultant to local industry (undergraduate school division) in the area

of computer applications in engineering.

1966 to 1969 Fritz Engineering Laboratory

Research Assisted in the design and testing of various types of

Assistant steel structures.

1966 North Dakota State Highway Department

Highway Responsible for construction surveying.

1965 U.S. Forest Service

Assistant Assisted in surveying responsibilities.

SECTION 2 PIPE PILE CAPACITY CURVES

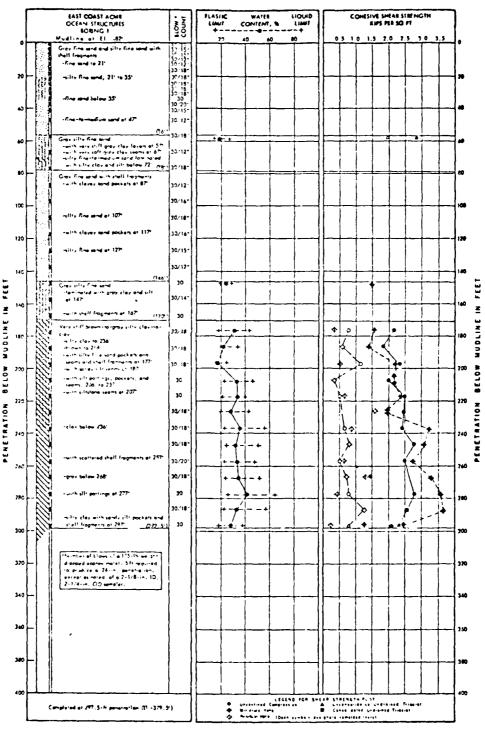
2.1 INTRODUCTION

Axial capacity curves are developed hereinafter for 42" diameter piling at boring site Nos. 2, 3A and 4, respectively. Axial capacity curves for boring site No. 1 were developed in previous report (Report No. 27-771-92, Appendix C, THREE-PILE CONCEPT CALCULATIONS) and are presented herein.

The method utilized for capacity curve development is an empirical procedure, as presented in the McClelland soils report, for pipe piles penetrating through alternating strata of sand and clay. The capacity curves for 30" diameter piling as given in the McClelland soils report are also included to illustrate the compatibility of those developed in this section.

2.2 CAPACITY CURVES FOR BORING SITE NO. 1

By C. Chern client U.S. NAVY __ subject Structural Concept Analysis (3-pite)
Date 3-31-76 Job No. 27-771-92 _ calculation lips Pilo Capacity Curres _
BORING # 1

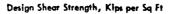


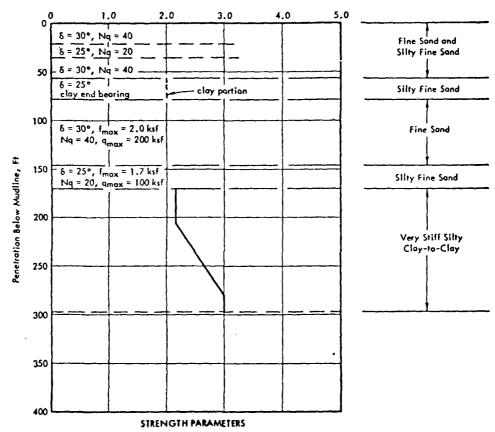
LOG OF BORING AND TEST RESULTS

Sheet 2.04 78_

By C. Chern client U.S. NAUY _ subject Struttural Concept Analysis (3-pile)

Date 3-31=76 Job No. 27-771-92 _ calculation Pipe Pile Capacity Curres _



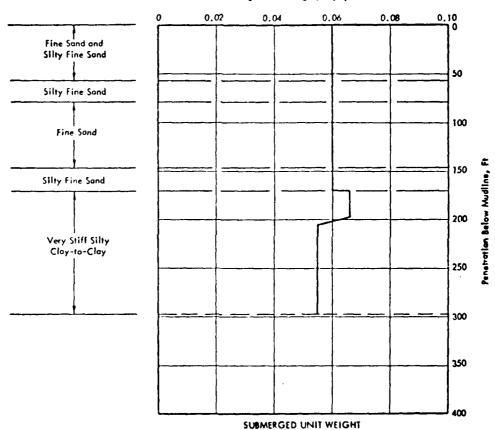


Sheet 2.05 of 78__

By C. Chern client U.S. NAVY _ subject Structural Concept Analysis (3:pite)
Date 3=31-76 Job No. 27-771-92 _ calculation Pipe Pile Capacity Curves.

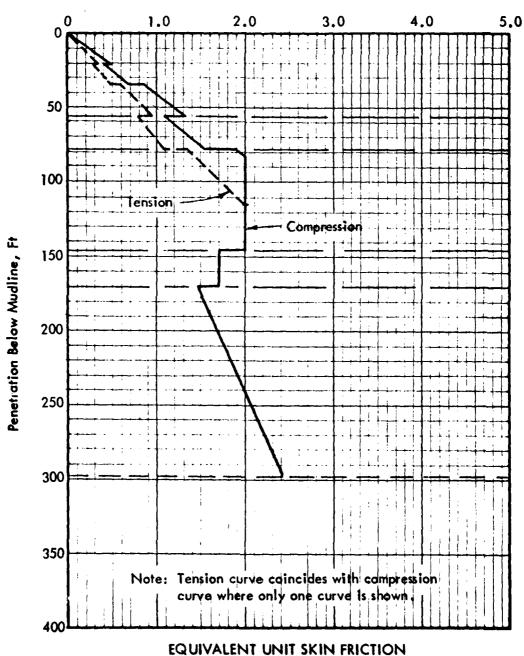
BORING #1





By C. Chern Client U.S. NAVY __ subject Structural Concept Analysis (3-pile)
Date 3=31-26 Job No. 27-771-92 _ calculation Pipe Pile Capacity Curves ___ BORING #1

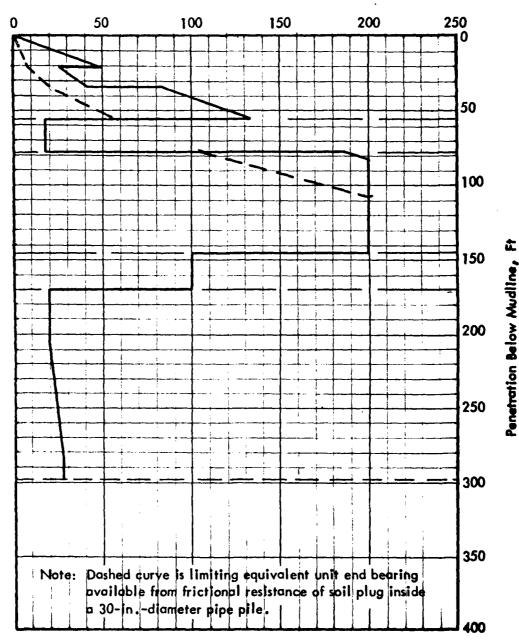
Equivalent Unit Skin Friction, Kips per Sq Ft



By C. Chern Client U.S. NAVY _ subject Structural Concept Analysis (3-pile)
Date 3=31-76 Job No. 27-771=92 _ colculation Pipe Pile Capacity Curves _ _ _

BORING #1

Unit End Bearing, Kips per Sq Ft



UNIT END BEARING

...

Sheet 2.08 of 78__

By C. Chern client U.S. NAUY __ subject Structural Concept Analysis (3-pile)
Date 3-3 L-76 Job No. 27-771-92 _ calculation Pipe Pile Capacity Curves ___
BORING # 1

PENETRATION BELOW MUDLINE		UNIT SKIN FRICTION	UNIT END BEARING	SEGMENT LENGTH
FT		KSF	KSF	FT
	0	0	0	
20		0.45	9	20
	20	0.40	9	
35		0.67	20	15
1	35	0.85	20	
56		1,30	55	21
ŀ	56	1.10	18	
79		1.65	18	23
]	79	1.90	105	
82		2.00	115	3
	82	2.00	115	
107		2.00	200	25
	107	2.00	200	
115		2.00	200	8
ļ	115	2.00	200	
145		2.00	200	30
	145	1.70	100	
170		1.70	100	25
	170	1.48	20	26
205		1.72	20	35
	205	1.72	20	74
280		2.30	28	75
	280	2.30	28	20
300	,	2.40	28	20

2=300 ft

UNIT CAPACITY IN COMPRESSION

Sheet 2.09 of 78__

BORING #1

, ,	PENETRATION UNIT SKIN BELOW MUDLINE FRICTION		UNIT END BEARING	SEGMENT LENGTH
FT		KSF	KSF	FT
20	0	O, 33	_	20
35	20	0.29 0.46	_	15
56	35	0.46	_	21
79	56	0.80	_	23
82	79	1.43		3
107	28	2.00		25
115	107	2.00		8
145	115	2.00	_	30
170	145	1.70		25
205	170	1.72		35
280	205	1.72	<u>-</u>	75
300	280	2.30	-	20

£300 ft

UNIT CAPACITY IN TENSION

Sheet 2.10 of 78__

By C-Chern client U.S. NAUY __ subject Structural Concept Analysis (3-pile)
Date 3=31-76 Job No. 27-77L-92 _ calculation Pipe Pile Capacity Curves _

Skin Friction Capacity (Qs = fas As)

--- Compression --- 0.D. = 42"

Basing #1 4 = 777/10

BORING #1

As = TD (al) = 10.996 (al) SQ.FT

					
Penetrotion	Unid Skin	Ave. Unitskin	Segmont	Skin Friction	Total
Below Midling	Friction	Friction	Lougth	in Segment	skinFrictim
(ft)	(ksf)	fas (KSf)	(Dl) (FT)	(Kips)	(kips)
0	0	0.225	20	49.5	0
20	0.45		20	41.7	49.5
20	0.40	0.535	15	000	
35	0.67			88.2	/37.7
35	0.85	1 075	- 1	0.4.00	
56	1.30	1.075	21	248.2	385.9
56	1.10				
79	1.65	1.375	23	347.7	733.6
79	1.90		_		
82	2.00	1.950	3	64.3	797.9
82	2.00				
107	2.00	2.00	25	549.8	1,347.7
107	2.00				17.7
115	2.00	2.00	8	175.9	1,523.6
115	2.00				7,723.0
145	2.00	2.00	30	659.8	2,183.4
145	1.70				2/1027
170		1.70	25	467.3	2,650.7
170	1.70				2,0%./
	1.48	1.60	35	615.8	20115
205	1.72				3,266.5
205	1.72	2.01	75	1.657.6	4 0 4 4
280	2.30				4,924.1
280	2,30	2.35	20	516.8	r 4400
300	2,40				5,440.9
1			5+	}	
			Z= 300 ft		
	}			1	
		<u> </u>			

Sheet 2.11 of 78__

By C. Chern client U.S. NAUX __ subject Structural Concept Analysis (3-pite):
Date 3-31-76 Job No. 22-771-92 calculation Ripe Pale Capacity Curves __
BORING # 1

End Bearing Capacity (Qp = 9 Ap)

		25"1	36"4	39"4	42"¢
Penetration	Unit End Bearg	3 <i>0"</i> \$	36" /		· , ,
BelowMudline	8	Ap	Αp	Ap	A-p
(ft)	(ksf)	4.915Q.FT	7.07 SQ.FT	8.30 SQ.FT	9.625Q.FT
0	0	0	0	0	0
20	9	44.2	63.6	74.7	86.6
20	9				. }
35	20	98.2	141.4	166.0	192.4
35	20				
56	35	171.9	247.5	290.5	336.7
56	18	88.4	127.3	149.4	173.2
79	18				
79	105	515.6	742.4	871.5	1,010.1
82	115	564.7	813.1	954.5	1,106.3
82	115	•			
107	200	982.0	1,414.0	1,660.0	1,924.0
107	200	_			
115	200				
115	200				
145	200				
145	100	491.0	707.0	8 30.0	962.0
170	100				
170	20	98.2	141.4	166.0	192.4
205	20	,0	}	1	
205	20		1		
	3	137.5	198.0	232.4	269.4
280	28	15/.5	176.0	-,0,1	201.4
	28			{	
300	1 60	{			
				}	}
<u> </u>	 	L	 		L

By C. Chern Client U.S. NANY _ subject Structural Concept Analysis (3-pile)
Date _3-31-76 Job No. 27-771-92 _ calculation Lipe Pile Capacity Curles _
BORING #1

Ultimate Pile Capacity (Q = Qs + Qp) --- Compression

Penetration	30°¢	36"P	39"Þ	42"¢
Below Mudline	Q	Ø	Q	Q
(ft)	(Kips)	(kips)	(Kips)	(kips)
0	0	0	0	0
20	79.5	106.0	120.6	136.1
20				
35	196.7	259.4	293.8	330.1
35				,
56	447.7	578.3	648.8	722.6
56	364.2	458.1	507.7	<i>55</i> 9.1
79	612.6	756.2	830.6	906.8
79	1,039.7	1,371.3	1,552.7	1,743.7
82	1,134.8	1,497.1	1,695.4	1,904.2
82				
107	1,944.8	2,569.3	2,911.4	3,271.7
107				
115	2,070.5	2,720.1	3,074.8	3,447.6
115				
145	2,541.7	3,285.6	3,687.4	4,107.4
145	2,050.7	2,578.6	2,857.4	3,145.4
170	2,384.5	2,979.2	3,291.3	3,612.7
170	1,991.7	2,413.6	2,627.3	2,842.7
205	2,431.5	2,941.4	3,199.1	3,458.5
205				
280	3,654.8	4418.8	4,804.7	5,193.5
280				
300	4.023.9	4,861.8	5,284.4	5,710.3

By C. Cherriclient Cl. S. NONY _ subject Structural Concept Analysis (3-pil)
Date 4 = 1-76 Job No. 27-72/-92 _ calculation Lipe Pile Capacity Curves _
BORING # 1

<u>Design Pile Capacity</u> (Qa= Q/F.S.) --- Compression

F.S. = 1.5

Penetr		30"¢	36°P	39"Ø	42"¢
Below Mudline		Qa	Qa	Q _d	Q _d
(+	t)	(kips)	(K 1 PS)	(KIPS)	(KIPS)
0		0	O	0	0
	20	53.0	70.7	80.4	90.7
20	35	131.1	172.9	195.9	220.1
35	56		385.5	432.5	481.7
-	- 76	298.5	305.4		
56	79	242.8 408.4	504.1	338.5 553.7	372. 7 604.5
79		693.1	914.2	1,035.	1.162.5
	82	756.5	998.1	1.130.3	1,269.5
85	1			1.04.0	- 101
	107	1,296.5	1,712.9	1,940.9	2,181.1
107	115	1,380.3	1,813.4	z,049.9	2,298.4
115					
	145	1,694.5	2,190.4	2,458.3	2,738.3
145		1,367.1	1.719.1	1,904.9	2,096.9
	170	1,589.7	1,986.1	2,194.2	2,408.5
170		1,327.8	1,609.1	1,751.5	1.895.1
	205	1,621.0	1,960.9	2,132.7	2,305.7
205	280	2 12/5	7 24 5 0	2 2 2 2	2 4 62 2
- 00	280	2,436.5	2,945.9	3,203.	3,462.3
280	300	2,682.6	3,241.2	3,522.9	3,806.9
	-				

Sheet 2.14 of 78__

By C. Chern client U. S. NAUX __ subject Structural Corcept Analysis (3-pile)
Date 3=31-76 Job No. 27-771-92 _ calculation lipe lile Capacity Corres _

Skin Friction Capacity (Qs = fas As)

--- Tension

6

O.D. = 42''

BORING#1

 $As = \pi D(\Delta l) = 10.996(\Delta l)$ SQ.FT

Penetration	Uniat Skin	Ave. Unid Skin	Segment	Skin Friction	Total
BelowMudling	Friction	Friction		in Segment	SkinFrictim
(ft)	(ksf)	fas (KSF)	(DL) (FT)	(Kipi)	(kips)
0	0	0.165		[0
20	0,33	0.100	20	36.3	36.3
20	0.29	0.375	15		
35	0.46	0.375		61.9	98.2
35	0.60	A 775	- 1	0	
56	0.95	0.775	21	179.0	277.2
56	0.80	0 05 0	2.7		
79	1.10	0.950	2.3	240.3	517.5
79	1.35	1.390	•	450	
82	1.43	1.570	3	45.9	563.4
82	1.43				
107	2.00	1.715	25	471.5	1,034.9
107	2.00	2 00	C		
115	2.00	2.00	8	175.9	1.210.8
115	2.00		_		
145	2.00	2.00	30	659.8	1,870.6
145	1.70	1.70			
170	1.70	1.70	25	467.3	2,337.9
170	1.48	1 (0		4 . 4 . 6	
205	1.72	1.60	35	615.8	2,953.7
205	1.72	2.0(
280	2.30	2.01	75	1,657.6	4,611.3
280	2.30	0.05		1./ 0	
300	2.40	2.35	.50	516.8	5,128.1
1			Z= 300 ft		
					ł
1					}

. .

Sheet 2.15 of 78__

BORING #1

<u>Ultimate</u> Pile Capacity (Q=Qs) --- Tension

Penel	ration	30° Þ	3 6"P	39"P	42"Ø
Below	Mudline	Q	Q	Q	Q
(F	(†)	(KIPS)	(KIPS)	(KIPS)	(KIPS)
0		0	0	0	0
<u> </u>	20	25.9	31.1	33.7	36.3
20	35	70.1	84.1	91.1	98.2
35	56	197.9	237.5	257.3	277.2
56	79	369.5	443.4	480.4	517.5
79	82	402.3	482.7	523.0	563.4
82	107	739.0	886.8	960.8	1,034.9
107	115	864.7	1,037.6	1,124.2	1,210.8
115	145	1,335.9	1,603.1	1,736.8	1.870.6
145	170	1,669.7	2,003.7	2,170.7	2,337.9
170	205	2,109.5	2.531.5	2.742.5	2,953.7
205	280	3,293.5	3,952.3	4,281.7	4,611.3
280	300	3.662.6	4,395.3	4.761.6	5, 128.1
				7.751.0	2) 1-0.1

By C. Chern client U.S. NAUX _ subject Structural Concept Analyzes 3-pills.
Date 4-1-76 Job No. 27-771-92 calculation Pripe file Capacity Curves
BORING #1

Design Pile Capacity (Qd = Q/F.S.) --- Tension

F.S. = 1.5

Penetration	30°P	36"¢	39"ø	42"¢
Below Mudline	Qd	Qd	Qa	Qa
(Ft)	(KIPS)	(KIPS)	(kips)	(KIPS)
0	0	0	0	0
20	17.3	20.7	22.5	24.2
20 .	46.7	5 / 1	(- 7	6 5. 5
	40.7	56.1	60.7	07.5
35 56	131.9	158.3	171.5	184.8
56				
79	246.3	295.6	320.3	345.0
79		2010	2497	375 (
87	268.2	321.8	348.7	375.6
82	492.7	591.2	640.5	689.9
107				
115	576.5	691.7	749.5	807.2
115				
145	890.6	1,068.7	1,157.9	.1,247.
145	1,113.1	1.335,8	1,447.	1,558.6
170			12.1.1.1	
205	1,406.3	1,687.7	1,828.3	1.969.1
205			_ ^	
280	2,195.7	2,634.9	2,854.5	3,074.2
280	2,441.7	2,930.2	3,174.4	3,418.7
			 	L

Sheet 2.17 of 78__

By C. Chern Client U.S. NAVY _ subject Structural Concept Analysis B-pile)
Date 4-1-76 Job No. 27-721-92 _ calculation Pipe Pile Capacity Curves __
BORING #1

42-in Diameter Pipe Piles

(i) At Penetration 45.5ft (=56-10.5')

End Bearing $Q_p = 46 \times 9.62 = 442.5 \text{ kips}$

Skin Friction $Q_s = 137.7 + 1.075(21 - 10.5) \times 10.996$ (see 8.2.17) = 137.7 + 124.1

= 261.8 kips

Ultimate Capacity Q = 261.8 + 442.5 = 704.3 kips Design Capacity Qd = 469.5 kips

(ii) At Penetration 134.5 ft (=145'-10.5')

End Bearing Op = 200 x 9.62 = 1,924 kips

Skin Friction $Q_s = 1.523.6 + 2.0x(30-10.5)x10.996$ (See Pg. 2.17) = 1.523.6 + 428.8

= 1,952.4 Kips

Ultimate Capacity Q = 1,952.4 + 1,924 = 3,876.4 kips Design Capacity Qd = 2,584.3 kips

By C. Chern client U.S. NAUX __ subject Structural Concapt Analysis Beple Date 4-1-76 Job No. 27-771-92 _ calculation Pipe Pile Capacity Curves __ BORING #1

(iii) At Penetration 159.5 ft (=120'-10.5')

End Bearing $Qp = 100 \times 9.62 = 962$ kips

Skin Friction $Q_s = 2,183.4 + 1.70 \times (25-10.5) \times 10.996$ (See P. 2.17)

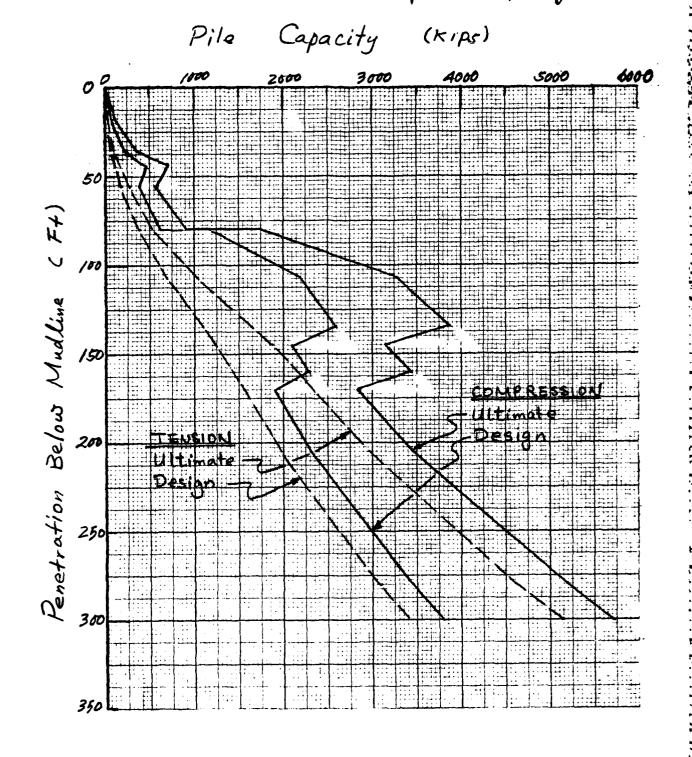
=2,183.4+271.1

= 2,454.5 kips

Ultimate Capacity Q = 2,454.5+962 = 3,416.5 Kips

Design Capacity Qd = 2,277.7 kips

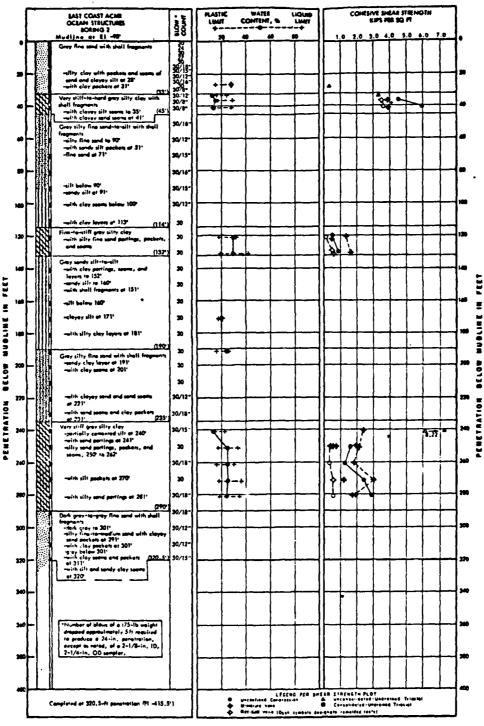
By C. Chern Client U. S. NAVY _ subject Structural Concept Analysis (3-pil)
Date 4=1=76_ Job No. 27-771-92_ calculation Pripe Pile Capacity Curves_



42-in. Diameter Pipe Piles (Boring #1)

2.3 CAPACITY CURVES FOR BORING SITE NO. 2

By S. Chern client U.S. NAUY __ subject Foundation Analysis _____ Date 6-1-76 Job No. 27-771-97 calculation Pipe Pile Capacity Conver __ BORING #2



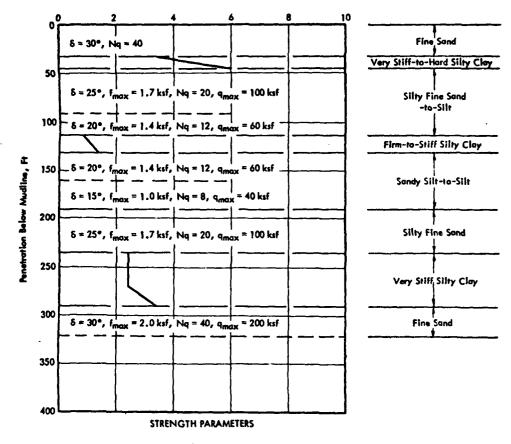
LOG OF BORING AND TEST RESULTS

Sheet 2.22 of 78__

By C. Chern client U.S. NAUY __ subject Foundation Analysis ______
Date 6-3-76_ Job No. 22-77/-97_ calculation Pipe Pile Capacity Curves ____

Boring #2

Design Shear Strength, Kips per Sq Ft

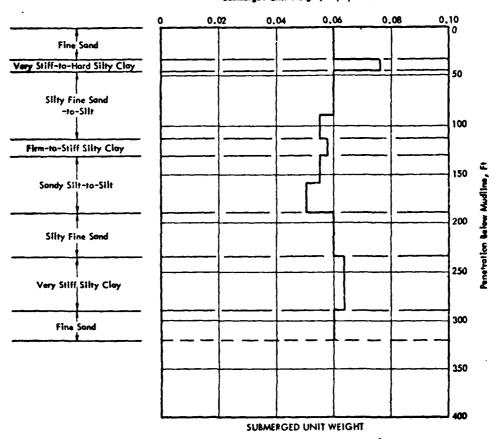


Sheet 2.23 of 78__

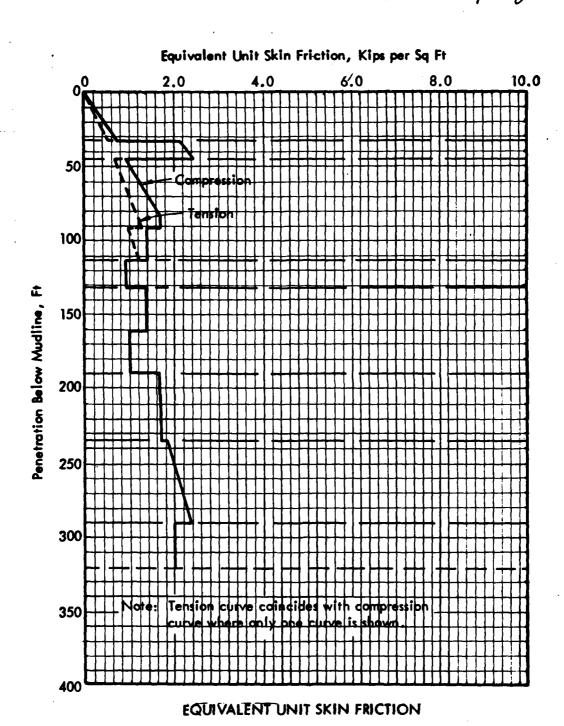
By C. Chern Client U.S. NAUY ___ subject Fow Action Analysis ____ Date 6=3-ZE Job No. 2Z-7Z1-9I _ calculation Ripefile Capacity Curves ___

Boring #2

Submerged Unit Weight, Kips per Cu Pt

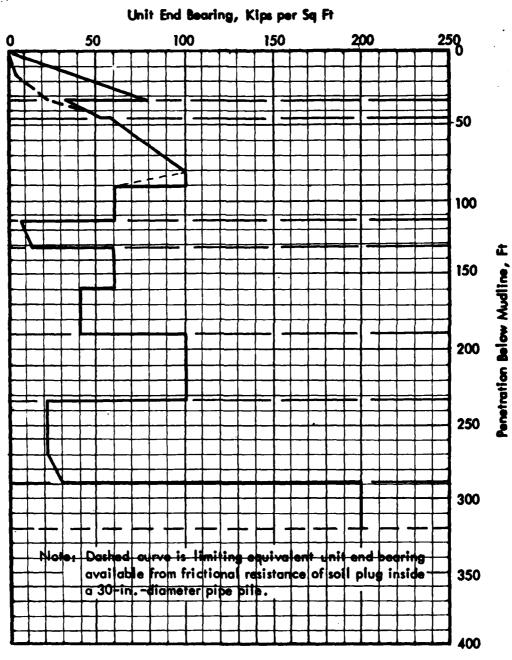


By C. Chern client U.S. NAVY __ subject Foundation Analysis ____ Date 6=1=76 Job No. 27-77L-97_ calculation Pipe Pile Capacity Curves __



Boring#2

By C. Chern client U.S. NAUX __ subject Foundation Analysis ____ Date 6 = 1-ZE Job No. 27 = 77/= 97 calculation Pipe Pile Capacity Curves __



UNIT END BEARING

Sheet 2.26 of 78_

By C. Chern client U.S. NAUX __ subject Foundation Analysis _____ Date 6= L-ZE Job No. ZZ-771 = 97 calculation Pipe Pile Capacity Corvec __

Boring #2

PENETR	MOITA	UNIT S	ŻKIN	UNIT END	SEGMENT
BELOW	MUDLINE	FRICT	101	BEARING	LENGTH
Ŧ	Т	KS	F	KSF	FT
	0		0	0	15
15		0.30		4.0	
	15		0.30	4.0	18
33		0.70		24.0	-
	33		2.15	24.0	7
40		2,27		45.0	<u> </u>
	40	- 4 -	2,27	45.0	5
45		2,40		53.0	
1	45		0.90	58.0	37
82		1.70		100.0	ļ
	82		1.70	100.0	9
91			1.70	100.0	
	91		1.40	60.0	23
114			1.40	60.0	
	114		0.90	7.0	18
132			0.90	13.0	
	132		1.40	60.0	28
160			1.40	60.0	
	160	!	1.00	40.0	30
190			1.00	40.0	-
	190	1	1.62	100.0	45
235		1.70		22.0	
	235		1.80	22.0	35
270		2.20		22.0	
300	270		2,20	22.0	20
290		2.35		30.0	
7.0	290		2.00	200.0	30
320			2.00	200.0	

£ 320 FT

Sheet 2.27 of 78_

By C. Chern client LLS. NAVY __ subject Foundation Analysis ____ Date 6-1-76 Job No. 27-771-97 _ calculation Ripe Pile Capacity Curves __

Boring #2

PENETRATION	UNIT SKIN	UNIT END	SEGMENT
BELOW MUDLINE	FRICTION	BEARING	LENGTH
FT	KSF	KSF	FT
15	0.20	_	15
33	0.20 0.50	_	18
33 40	2.15 2.27	-	7
40	2.27	-	5
45 82	0.70 0.44	_	37
82 91	0.44	_	9
91	1.00		23
114	0.90 0.90	_	18
160	1.40 1.40	_	28
190	1.00 1.00	-	30
190 235	1.65 1.70	-	45
235 270	1.80 2.20	-	35
270 290	2.20 2.35	-	20
290 320	2.00 2.00	-	30

Z320 FT

UNIT CAPACITY IN TENSION

Sheet 2.28 of 78__

By C. Chern_ client U.S. NAUY ___ subject Foundation Analysis ____ Date 6-1-76_ Job No. 27-771-91_ calculation Ripefile Capacity Curves ___

SKIN FRICTION CAPACITY (Qs = fas As)

-- COMPRESSION --

O.D. = 42"

(Boring #2)

 $A_s = \pi D(\Delta L) = 10.996(\Delta L)$ SQ. FT

PENETRATION	UNIT SKIN	Ave. Unit Skin	SEGMENT	SkIN FRICTION	TOTAL SKIN
BELOW MUDLINE	FRICTION	FRICTION (fas)	LENGTH (AL)	IN SEGMENT	FRICTION
FT	KSF	KSF	FT	KIPS	KIPS
15	0.30	0.15	15	24.7	24.7
15 33	0,30 0.70	0.50	18	99.0	123.7
33 40	2.15 2.27	2.21	7	170.1	Z93.8
40 45	2.27 2,40	2.33	5	128.1	421.9
45 82	o.90 1.70	1.30	37	528.9	950.8
82 91	1.70 1.70	1.70	9	168.2	1,119.0
91	1.40	1.40	23	354.1	1,473.1
114 132	0.90 0.90	0.90	18	178.1	1,651.2
132 160	1.40	1.40	28	431.0	2,082.2
160 190	1.00	1.00	30	329.9	2,412.1
190 235	1.62	1.66	45	821.4	3,233.5
235	1.80	2.00	35	7 69.7	4,003.2
270 290	2.20 2.35	2.27	20	499.2	4.502.4
320 290	2.00 2.00	2.00	30	6 59.8	5.162.2

Sheet 2.29 of 78__

By C. Chern Client U.S. NAUY __ subject Foundation Analysis ____
Date 6 = 1 = 76 _ Job No. 27-77/-97 _ calculation Pape Pile Capacity Curves ___
BORING #2

END BEARING CAPACITY (Qp = & Ap)

 $A_{P} = \frac{\pi}{4} (42)^{2}/144 = 9.62 \text{ SQ. FT}$

PENETRATION	UNIT END	
BELOW MUDLIN	E BEARING	END BEARING
FT	KSF	KIPS
0	0	0
15	4.0	38.5
15	4.0	
33	24.0	230.9
33	24.0	
40	45.0	432.9
40	45.0	
45	53.0	509.9
45	58-0	558.0
82	100.0	962.0
82	100.0	
91	100.0	
91	60.0	577.2
114	60.0	
114	7.0	67.3
132	13.0	125.
132	60.0	577.2
160	60.0	3010
160	40.0	384.8
190	40.0	0/5
190	100.0	962.0
235	10 0.0	
235	22.0	211.6
270	22.0	
270	22.0	2806
290	30.0	288.6
290 320	200.0	1,924.0
760		

Sheet 2.30 of 78_

By C. Chern Client U.S. NAVY ___ subject Foundation Analysis ____ Date 6-1-76_ Job No. 27-771-97_ calculation Ripa Pila Capacity Curves ____

PILE CAPACITY IN COMPRESSION (Boring #2)

PENETRATION BELOW MUDLINE	SKIN FRICTION	END BEARING	ULTIMATE CAPACITY	DESIGN CAPACITY (F.S.=1.5)
FT	KIP5	KIP5	KIPS	KIPS
0	0	0	0	0
15	24.7	38.5	63.2	42.1
33	123.7	230.9	354.6	2 36.4
33 40	293.8	43z.9	726.7	484.5
40	421.9	509.9	931.8	621.2
45	- 10	558.0	979.9	653.3
82	950.8	962.0	1,912.8	1,275.2
82				
91	1,119.0		2,081.0	1,387.3
91		577.2	1.696.2	1,130.8
114	1,473.		2,050.3	1,366.9
114		67. 3	1,540.4	1,026.9
132	1,651.2	[25.]	1,776.3	1.184.2
132		577.2	2,228.4	1,485.6
160	2,082.2		2,659.4	1,772.9
160		3 84.8	2,467.0	1,644.7
190	2,412.1		2,796.9	1,864.6
190		962.0	3,374.	2,249.4
235	3,233.5		4,195.5	2.797.0
235	- :-	211.6	3,445.1	2,296.7
270	4,003.2		4,214.8	2,809.9
270	4 4			
290	4,502.4	288.6	4,791.0	3, 194.0
290	r 1/2 -	1,924.0	6,426.4	4,284.3
320	5,162.2		7,086.2	4.724.

Sheet 2.31 of _78_

By C. Chern client U.S. NAUY __ subject Foundation Analysis _____ Date 6=1-76_ Job No. 27-771-97 _ calculation Ripe Pile Capacity Curves ____

SKIN FRICTION CAPACITY (Qs = fas As)

-- TENSION --

0.D. = 42"

BORING #2

As = 1 D(AL) = 10.996 (AL) SO.FT

PENETRA-	1	UNIT SKIN	AVE. UNITSKIN	SEGMENT	SKIN FRICTION	TOTALSKIN
BELOW M	UDLINE	FRICTION	FRICTION (fas)	LENGTH (al)	IN SEGMENT	FRICTION
FT		kSF	KSF	FI	KIPS	KIPS
15	0	0.20	0.10	15	16.5	0 16.5
33	15	0.20 0.50	0.35	18	69.3	85.8
40	33	2.15	2.21	7	170.1	255.9
45	10	2.27 2.40	2,33	5	128.1	384.0
82	15	0.70 0.44	0.57	37	231.9	615.9
91	32	0.44	0.84	9	83.1	699.0
114)	1.00 1.20	1.10	23	278.2	977.2
132	14	0,90 0,90	0.90	18	178.1	1,155.3
160	32	1.40	1.40	28	431.0	1.586.3
190	60	1.00 1.00	1.00	30	329.9	1,916.2
235	90	1.65 1.70	1.67	45	826.3	2,742.5
270	35	1.80 2.20	2,00	35	769.7	3.512.2
290	70	2.20 2.35	2.27	20	499.2	4,011.4
320	90	2.00 2.00	2.00	30	659.8	4,671.2

Sheet 2.32 of 78__

By C. Chern client U.S. NAUX ___ subject Foundation Analysis ____ Date 6-1-26 Job No. 27-771-97 _ calculation Pipe Pile Capacity Curves ___

PILE CAPACITY IN TENSION (Boring #2)

PENETRATION		
BELDW	ULTIMATE	DESIGN
MUDLINE	CAPACITY	CAPACITY
MUDEINE	GAPACI 17	(F.S.=1.5)
FT	KIPS	KIPS
0	0	0
15	16.5	11.0
15		
33	85.8	57.2
33		
40	255.9	170.6
45 40	2010	
promise to a temperature and a con-	384.0	256.0
82	615.9	410.6
82		
91	699.0	466.0
91	977.2	651.5
114		
132	1,155.3	770.2
132		
160	1.586.3	1,057.5
190	1,916.2	1,277.5
190		
235	2,742.5	1.828.3
235		
270	3,512.2	2,341.5
270	4 4	- 1 - 1 -
290	4,011.4	2,674.3
320	4.671.2	3,114.1

Sheet 2-33 of 78

By C. Cherric Client U.S. NAUY __ subject Foundation Analysis ____ Date 6=3-76 Job No. 27-771-97 _ calculation Experise Capacity Conves ___ Boring # 2

Modification on Unit End Bearing Capacity

42'' 3-D = 10.5 ft $A_p = 9.62$ SQ.FT

Boring #2

(i) At penetration 80.5 FT (=91'-10.5')

End Bearing $Q_p = (100 \text{ ksf}) \times (9.62) = 962 \text{ kips}$ Skin Friction $Q_s = 960.8 \text{ kips}$ (Taken at 82FT) Ultimate Capacity Q = 962 + 960.8 = 1,912.8Design Capacity $Q_d = 1,274.8 \text{ kips}$

(ii) At Penetration 103.5 FT (=114'-10.5')

End Bearing $Q_p = (60 \text{ KSF}) \times (9.62) = 577.2 \text{ KIPS}$ Skin Friction $Q_s = 1119.0 + 354.1 \times \frac{103.5 - 91}{23} = 1,311.4 \text{ KIPS}$

Ultimate Capacity Q = 577.2 + 1,311.4 = 1,888.6 KIPSDesign Capacity $Q_d = 1,259.1 \text{ KIPS}$

Sheet 2.34 of _78_

By C. Cheriz client U.S. NAUY __ subject Foundation Analysis ____
Date 6-3-ZE Job No. 27-771-97 _ colculation tipe Pile Capacity Curres __
BORING # 2

(iii) At penetration 142.5 FT (=132'+10.5')

End Bearing $Q_p = (60 \text{ KSF}) \times (9.62) = 577.2 \text{ KIPS}$

Skin Friction Qs = 1651.2+43/x 10.5 = 1,812.8 KIPS

Ultimate Capacity Q = 577.2+1,812.8 = 2,390.0 KIPS

Design Capacity Qd = 1,593.3 Kips

(iv) At Penetration 149.5 FT (= 160'-10.5')

End Bearing Qp = (60 KSF) x(9.62) = 577.2 KIPS

Skin Friction Qs = 1651.2+431 x 149.5-132 = 1,920.6 klps

Ultimate Capacity Q = 577.2 + 1,920.6 = 2,497.8 KIPS

Design Capacity Qs = 1,665,2 Kips

(v.) At Penetration 200.5 FT (=190410.5')

End Bearing $O_p = (100 \text{ ksF}) \times (9.62) = 962 \text{ kips}$

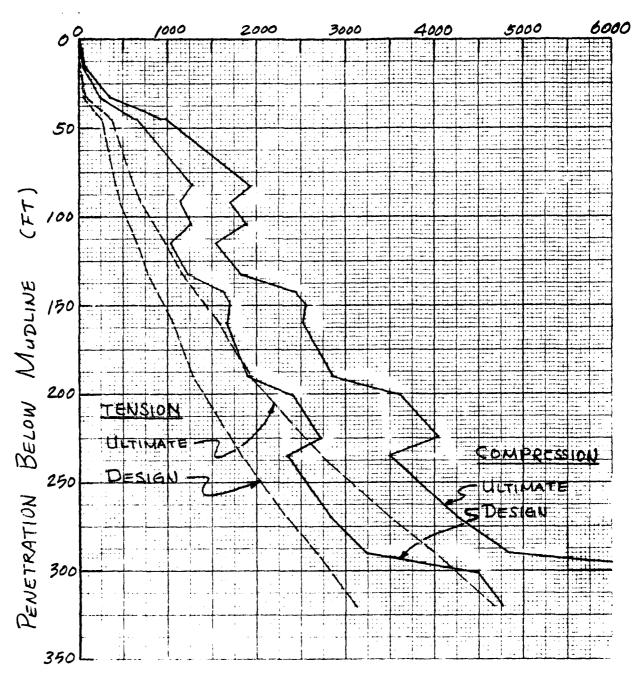
Skin Friction $Q_S = 2412.1 + 821.4 \times \frac{10.5}{45} = 2,603.8^{k1PS}$

Ultimate Capacity Q = 96z+2,603.8 = 3,565.8 KIPS

Design Capacity Qd = 2377.2

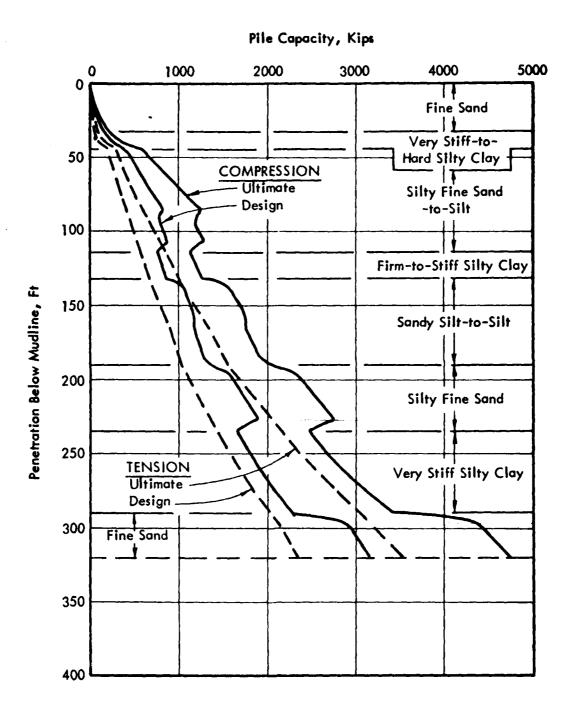
By C. Cherr Client U.S. NAUY __ subject Foundation Aringsis ____ Date 6-1-76 Job No. 27-771-97 colculation Pipe Pile Capacity Chrycs ___





42-IN. DIAMETER PIPE PILES (Boring #2)

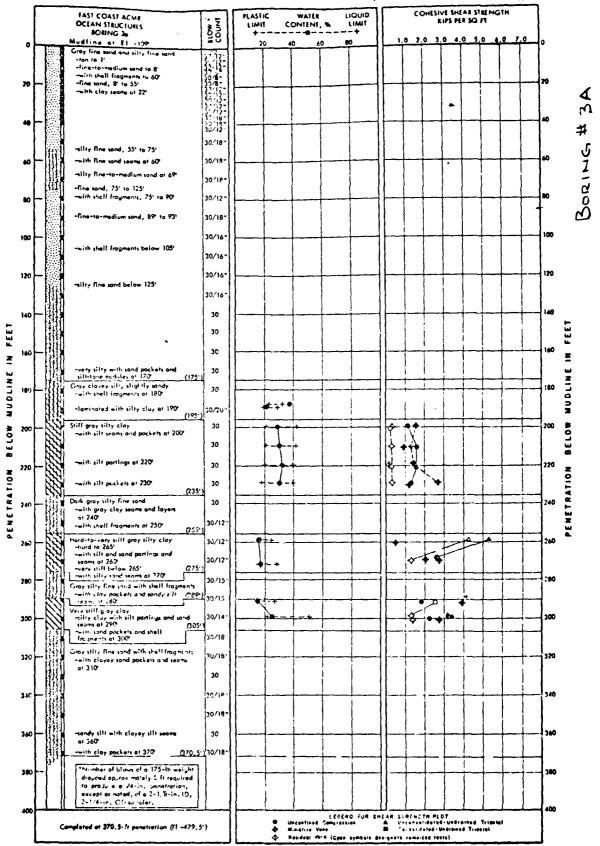
By C. Chern Client U.S. NAUX ___ subject Foundation Analysis ____ Date 6-3-76 Job No. ZZ-7ZL-97 _ calculation Ripe Pile Capacity Curves ___



PILE CAPACITY CURVES
30-in. Diameter Pipe Piles
Boring 2

2.4 CAPACITY CURVES FOR BORING SITE NO. 3A

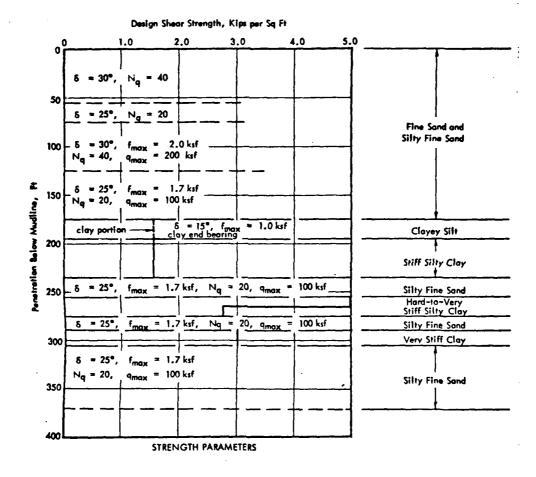
By C. Chern client U. S. NAVY _ subject Foundation Analysis ____ Date 6=1=76_ Job No. 27-771-97_ calculation Pipe Pile Capacity Conves



Sheet 2.42 of 78__

By C. Chern Client U.S. NAUX ___ subject Foundation Analysis____ Date 6=3-Z6 Job No. ZZ-7Z1-97_ calculation Pipefile Capacity Curves__

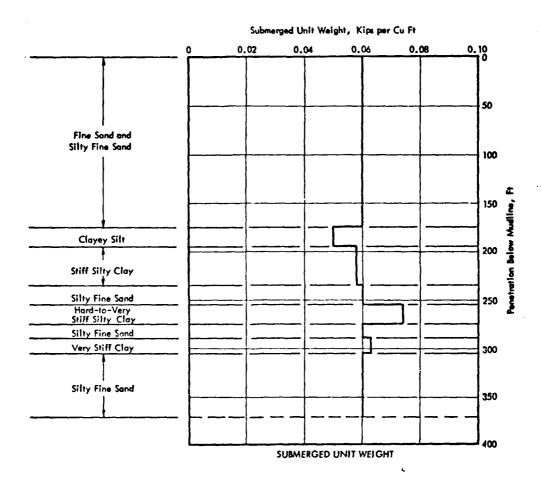
Boring #3A



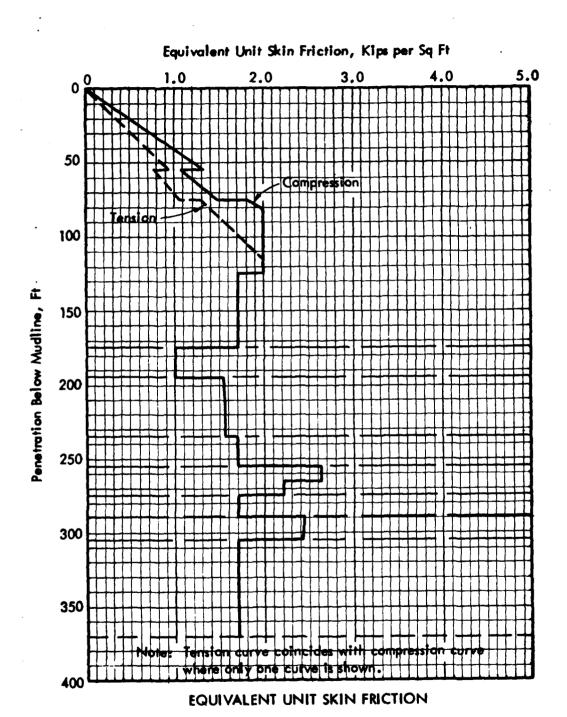
Sheet 2-41 of 78__

By C. Chern client LI.S. NAVY __ subject Foundation line lists _____
Date 6-3-Z6 Job No. 27-771-97 _ calculation Pipe Pile Capacity Curves __

Boring #3A



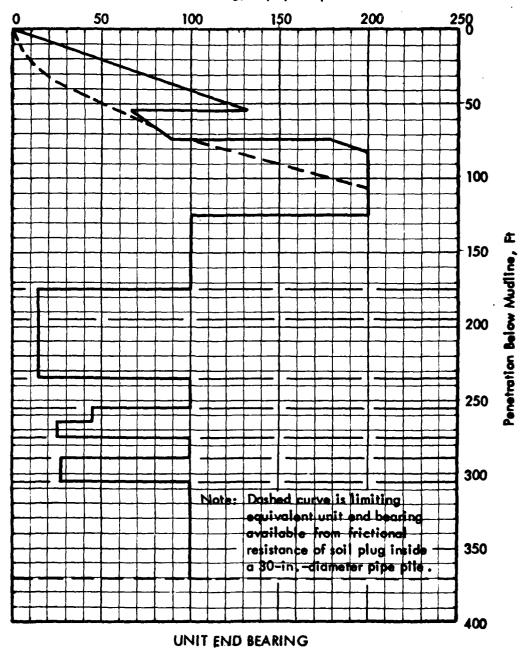
By C. Chern Client U.S. NAVY __ subject Foundation Analysis _____
Date 6=1=Z6 Job No. 27=771=97 Calculation Pipe File Capacity Curve! ___



Boring 3A

By C. Chern client U.S. NAVY __ subject Foundation Analysis ____ Date 6= L-76 Job No. 27-771-97 _ calculation Pipe File Capacity Curves __

Unit End Bearing, Kips per Sq Ft



Boring#3A

Sheet 2.44 of 78__

By C. Cherr- client U.S. NAVY __ subject Foundation Analysis ____
Date 6=1= ZE Job No. 27-771-27_ colculation Pipe Pile Capacity Curves ___

Boring #3A

PENETRATION BELOW MUDLINE	UNIT SKIN FRICTION	UNIT END BEARING	SEGMENT LENGTH
FT	KSF	KSF	FT
20	0.5	0	20
35 20	0.85	24.0	15
35 55	1.30	60.0	20
65	1.10	80.0	10
75 65	1.50	90.0	10
82 75	2.00	100.0	7
106	2.00	200.0	24
106	2.00	200.0	9
125	2.00	200.0	10
175	1.70	100.0 100.0	50
175	1.00	14.0	20
235	1.53	14.0	40
235 255	1.70	100.0	20
265 255	2.63 2.63	45.0 45.0	10
275 265	2.20	25.0 25.0	10
290 275	1.70 1.70	100.0	15
305 290	2.40	27.0 27.0	15
305 370	1.70	100.0	65
			2370

Sheet 2.45 of 78_

By C. Chern Client U.S. NAVY _ _ subject Foundation Analysis ____ Date 6-2-76_ Job No. 27-771-97_ calculation Pipe Pile Capacity Curred ___

Boring #3A

PENETRATION BELOW MUDLINE	UNIT SKIN FRICTION	UNIT END BEARING	SEGMENT- LENGTH
FT	KSF	KSF	FT
20	0.35	_	20
35	0.60	_	15
55 35	0.90	-	20
55 65	0.80	-	10
75 65	1.05	-	10
82 82	1.42	_	7
106	1.80	-	24
106	2.00	-	9
115	2.00	-	10
175	1.70	-	50
195	1.00	-	20
195	.1.53 1.57	-	40
235 255	1.70 1.70	-	20
255	2.63		10
265 265 275	2.20	-	10
290 275	1.70	_	15
305 290	2.40	_	15
305 370	1.70	_	65
,			2370

By C. Chexil client U.S. NAUX ___ subject Foundation Analysis _____
Date 6=2-76 Job No. 27-77L-97 calculation Bipe Pile Capacity Carnes _____

SKIN FRICTION CAPACITY (Qs = fo; As)

-- COMPRESSION -(Boring #3A)

0.D. = 42" As = 10 (D(D) = 10.996(A) SQ.FT

PENETRATION UNIT SKIN AVE. UNIT SKIN SEGMENT BELOW MUDLINE FRICTION FRICTION LENGTH (fas) (AL)	SKIN FRICTION IN SEGMENT (Qs) KIPS	TOTAL SKIN FRICTION
(tas) (AL)	(Qs)	FRICTION
1 1 1	1	
FT KSF KSF FT		KIPS
20 0.5 0.25 20	55.0	55.0
35 20 0.85 0.67 15	110.5	165.5
35 0.85 55 1.30 1.07 ZO	235.3	400.8
65 55 1.10 1.20 10	131.9	532.7
75 1.30 1.40 10	153.9	686.6
75 1.80 82 2.00 1.90 7	146.2	832.8
106 2.00 2.00 24	527.8	1,360.6
115 2.00 2.00 9	197.9	1,558.5
125 2.00 2.00 10	219.9	1,7784
175 1.70 1.7 50	934.7	2,7/3.1
195 1.00 1.0 20	219.9	2,933.0
235 1.57 1.55 40	681.8	3,614.8
235 1.70 1.70 20	373.9	3,988.7
265 2.63 2.63 10	289.2	4,277.9
275 2.20 2.20 10	241.9	4.519.8
290 1.70 1.70 15	280.4	4,800.2
305 290 2.45 2.42 15	399,2	5,199.4
370 305 1.70 1.70 65	1,215.	6,4145
₹370		

Sheet 2.47 of 78__

By C. Chern Client U. S. NAVY ___ subject Foundation Analysis __ Date 6-2-76 Job No. 27-771-97 _ calculation Ripe Pile Capacity Carres __ BORING # 3A

END BEARING CAPACITY (Qp= JAp) Ap = 9.62 SO.FT

		
PENETRATION	UNIT END	T D10
BELOW MUDLINE	BEARING	END BEARING
FT	KSF	KIPS
0	0	
20	10.0	96.2
20	_	
<u>35</u> 35	24.0	230.9
55	60.0	577.2
65 65	80.0	769.6
75 65	90.0	865.8
75	90.0	962.0
82	125.0	1,202.5
82	200.0	1,924.0
106	200.0	,724.0
115	200.0	
115		
125	200.0	_ ,
125	100.0	962.0
175	14.0	134.7
195		
235	14.0	
235	100.0	962.0
255	100.0	_ ,
265 265	45.0 45.0	432.9
265 265 275	25.0	240.5
275	25.0	962.0
290	100.0	
305	27.0 27.0	259.7
305 370	100.0	962.0

Sheet 2-43 of 78__

By C. Chern client U. S. NAUX __ subject Foundation Arialysis _____ Date 6-2-76 Job No. 27-77/-97 _ calculation Pipe Pile Capacity Curves ___

PILE CAPACITY IN COMPRESSION (Boring #34)

<u> </u>	 			
PENETRATION BELOW		END BEARING	ULTIMATE	DESIGN
Mud LINE			CAPACITY	(F.S.=1.5)
FT	KIPS	KIPS	KIPS	KIPS
0	O			
20	55.0	96.2	151.2	100.8
20				
35 35	165.5	230.9	396.4	264.3
55	400.8	577.2	978.0	652.0
55				
65	532.7	769.6	1,302.3	868.2
65 75 75	686.6	9/5 9	1,552.4	1,034.9
75	000.0	865.8 962.0	1,648.6	1,099.1
82	832.8	1,202.5	2,035.3	1,356.9
82 82				
106	1,360.6	1,924.0	3, 284.6	2,189.7
106	1 1 5 0 5		3, 482.5	2,321.7
115	1.558.5		2,402.2	2,361,7
125	1,778.4		3,702.4	2,468.3
125	ł	96z.0	2,740.4	1,826.9
1 1/2	2,713.1		3,675.1	2,450.1
175		134.7	2,847.8	1,898.5
195	2,933.0		3,067.7	2.045.
235 235 255	3,614.8		3, 749.5	2,499.7
235		962.0	4,576.8	3051.2
255	3,988.7		4,950.7	3,300.5
255 255 265	1	432.9	4.421.6	2,947.7
265 265 265	4,277.9	2405	4,710.8	3,140,5
275	4.519.8	240.5	4,518.4	3.012.3 3.17 3. 5
275	417171	962.0	4,760.3 5,481.8	3,654.5
290	4,800,2	, , , , ,	5,762.2	3,841.5
290	4	259.7	5,059.9	3,373.3
305	5, 199.4	Í	5.459.1	3,639.4
305 370	6,414.5	962.0	6,161.4 7,376.5	4.107.6
2/0	0,419.2		1,3/0.2	7.71/./

Sheet 2.49 of 78_

By C. Click of Client U.S. NAVY __ subject Foundation Analysis _____

Date 6-2-76 Job No. 27-771-97 _ calculation Pipe Pile Capacity Curves _
BORING #3A

SKIN FRICTION CAPACITY (Qs = fas As)
0.D. = 42"

-- Tension --

As = MD(al)=10.996(al) SO.PT

PENETRATION	UNIT SKIN	AVE. UNITSKIA	SEGMENT	SKN FRICTION	TOTAL SKIN
BELOW MUDLINE	FRICTION	FRICTION (fas)	LENGTH (AL)	IN SEGNAENT (Q_s)	FRICTION
FT	KSF	KSF	FT	KIPS	KIPS
20	0.35	0.17	20	37.4	0 37.4
35 20	0.35	0.47	15	77.5	114.9
35 55	0.60 0.90	0.75	70	164.9	279.8
65 55	0.80	0.86	10	94.6	374.4
75 75	0.92	0.98	10	107.8	482.2
75 82	1.30	1.36	7	104.7	586.9
82	1, 42 1,80	1.61	24	424.9	1,011.8
106	2.00	2-01.90	9	188.8	1.199.8
115	2.00 2.00	2.0	10	219.9	1,419.7
175	1.70	1.7	50	934.7	2,354.4
175	1.00	1.0	20	219.9	2,574.3
195 235	1.53	1.55	40	681.8	3.256.
235 255	1.70	1.70	20	373.9	3,630.0
265 265	2.63 2.63	2.63	10	289.2	3,919.2
265 275	2,20 2,20	2.20	10	241.9	4,161.
275 290	1.70	1.70	15	280.4	4,441.5
290 305	2,45 2,40	2.42	15	399.2	4,840.7
370 305	1.70	1.70	65	1,215.	6.055.8
			≥370		



Sheet 2.50 of 78_

By C. Chern Client U.S. NAUY __ subject Foundation Analysis ____ Date 6-2-76_ Job No. 27-771-97 _ calculation Pipe Pile Capacity Curves ___

PILE CAPACITY IN TENSION (Boring #3A)

PENETRATION						
BELOW	ULTIMATE	DESIAN				
MUDLINE	CAPACITY	CAPACITY				
		(F.S.=1.5)				
FT	KIPS	KIPS				
20 ° zo	0 37.4	24.9				
20 35 35	114.9	76.6				
35 55 55	279.8	186.5				
65 65	374.4	249.6				
75 75	482.2	321.5				
82 82 82	586.9	391.3				
106	1,011.8	674.5				
106	1,199.8	799.9				
125	1.419.7	946.5				
125 175 175	2, 354.4	1.569.6				
175 195 195	2,574.3	1,716.2				
195 235 235	3,256.	2,170.7				
255 255 255	3,630.0	2,420.0				
265 265 265	3,919.2	2,612.8				
275 275	4,161.1	2,774.1				
275 290 290	4.441.5	2,961.0				
1 205	4,840.7	3,227.0				
305 370	6,055.8	4,037.2				

sheet 2.51 of 78_

By C. Chern Client U.S. NAVY ___ subject Foundation Analysis ____ Date 6-3-76 Job No. 27-771=97 _ calculation Ripe Pile Capacity Curves ___ BORING # 3A

Modification on Unit End Bearing Capacity

Boring #3A

(i) At penetration 114.5" (=125'-10.5')

End Bearing $Q_p = (200 \text{ KSF}) \times (9.62) = 1.924 \text{ KIPS}$

Skin Friction Qs = 1,558.5 Kips (Taken at 115 T)

Ultimate Capacity Q = 1,924+1,558.5 = 3,482.5 KIPS

Design Capacity Qd = 2,321.7 KIPS

(ii) At Penetration 164.5 FT (=175'-10.5')

End Bearing $Q_p = (100 \text{ KSF}) \times (9.62) = 962 \text{ KIPS}$

Skin Friction Qs = 1,778.4+934.7 × 164.5-125 = 2,516.8 kips

Ultimate Capacity Q = 962+2,516.8 = 3,478,8 KIPS

Design Capacity Q1 = 2,319.2 KIPS

(iii) At Penetration 245 FT (= 235'+10')

End Bearing $O_p = (100 \text{ KSF}) \times (9.62) = 962 \text{ kips}$

Skin Friction Qs= 3.614.8+ 2 x 374 = 3,801.8 KIPS

Ultimate Capacity Q = 962+3,801.8 = 4,763.8 KIPS

Design Capacity Qd = 3,175.9 KIPS

Sheet 2.52 of 78_

By C. Cherry Client LIS NAUX ___ subject Foundation Analysis ____
Date 6-3-76 Job No. 27-771-97 _ calculation Lipe Pile Capacity Curves ___
BORING # 3A

(iv) At Penetration 315.5 FT (=305'+10.5')

End Bearing

Op = (100 KSF) × (9.62) = 962 Kips

Skin Friction

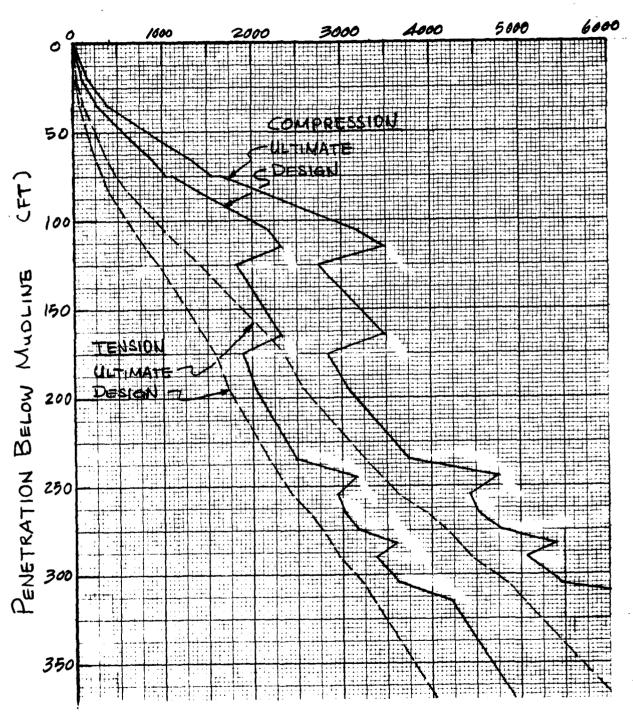
 $Q_s = 5.199.4 + 1.215.1 \times \frac{10.5}{65} = 5.395.7 \text{ KIRS}$

Ultimate Capacity Q = 962+5,395.7 = 6,357.7 KIPS

Design Capacity Q1 = 4,238.5 KIPS

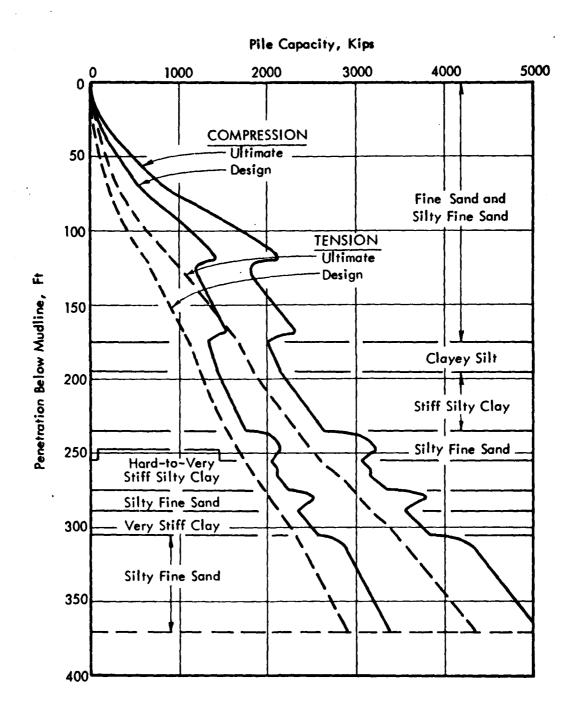
By C. Chern client U.S. NAUX __ subject Foundation Anaysis ____ Date 6-2-76_ Job No. 27-771-91_ calculation Pipe Pile Capacity Curves ___

PILE CAPACITY (KIPS)



42-IN. DIAMETER PIPE PILES
(Boring # 3A)

By C. Chern Client U.S. NAYY ___ subject Foundation Analysis ____ Date 6=3-76 Job No. 27-77L-97 _ calculation Pipe Pile Capacity Curves _



PILE CAPACITY CURVES
30-in. Diameter Pipe Piles
Boring 3a

2.5 CAPACITY CURVES FOR BORING SITE NO. 4

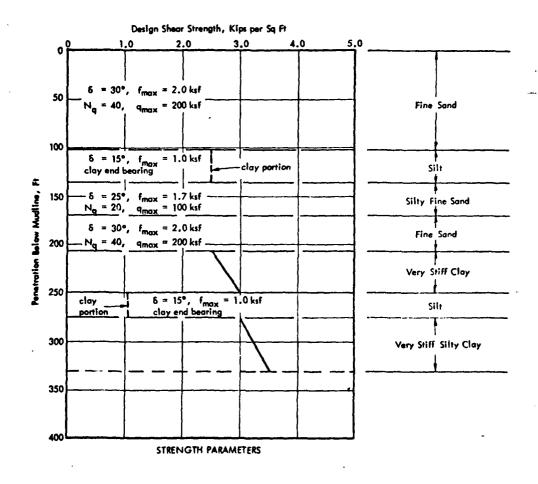
By C. Chern CHEAT CHEAT U.S. NAVY __ subject Foundation Analysis _____ Date 6-2-76 Job No. 27-771-97 _ calculation Pipe Pile Capacity Curves_

	<u> </u>	EAST COAST ACME	: 5	PLAST		WATER		LIQUID		C	OHESIN	E SHEAR	5106.0	0	- .			
	ł	OCEAN STRUCTURES BORING 4 Mudline et El -108	MON COUNT	+-		ONTENT		*-+				3 .2.0	~ .,		1			
•		Ten-te-gray fine sand		 	<u> </u>	<u>" </u>	Ī	1	<u> </u>			Ï	7.3	0.0	Ч.			
		-with shell fragments to 14' -ten to 3' -gray, 3' to 27'	10000000000000000000000000000000000000	ļ		l	}					1		1 1	- 1			
30	-	-shell fragment layer at 10"	30/15	├		├	├	 -	 	╁─	-	-+	+-	╁╌┤	-1	×		
		-silty fine sand at 13.5' -with shall fragments, 21' to 38'	38/13:	}	ł	}	l				} }	1	İ					
		-fine-to-medium sand at 25' -tan, 27' to 32'	30/15- 30/18-		<u> </u>	↓	 	 		<u> </u>		_	4-	-		44		4
		-gray below 32' -with clay pockets at 50'		l	ł	ł	{	1		1								ال فلاس
			30/15-	ľ	1	1	1		11.				1					- ₹
40		-with shell fragments, 60' to 71' -laminated gray sand, dark gray sand and dark gray silty clay at 61'	30/12-			-						*				46		ર્
		-with clay packets at 71'	30/15*	ĺ	1	1	ĺ	1	[[- 1	ľ					2
90	F.	-silty fine sand at 81'	30/18-		1	 	1	+		\vdash		_	1	1		80		3021NG
			30/18-	j		1		1	[[İ		- {		1				0
100	 - 		30/18*	<u> </u>	=	 	 	+	=	 -		-	= =	+-	Ħ	100		M
	1 11111	Gray silt with silty clay packets	30/18-]				1							, ,	1		
120	-	at 111' —sity clay with shall fragments and and clay seams at 121'	30	 	1 3=	<u> </u>	ļ	-	-	┼			-	+	\vdash	120		
3.		-with sandy silt seams and clay	30			_												
140		pockets at 131' (136' Gray silty fine sand with shell fragments	1 1	_	<u> </u>	<u> </u>	<u> </u>	+		<u> </u>			\pm	1		140		
		-with fine-to-medium sand seams and gravel at 141*	30				l	1				.		1		1	E3	
FEET		-with clay packets at 151'	30/12*			1		i	11	}		1		1		ļ	FEE	
₹ 160	1		30/12-		T-		1	1								140	Ī	
		(170° Tan-to-gray fine sand with shell fragments	30/12"	-	 	 	+	†		 	-	-	+	1		1	E E	
UDLINE		-ran to 180' -with clayey sand pockets at 171'	30/12-		┼─	 	┼		(├	╁─			+-	+-		190		
2 2		-gray below 180* -very hard drilling below 180*	30/12*			ļ	1		[[İ				İ	2	
≥ 200	 - 		30/10"		 	┼				┼		-	+	+	-	500	¥07	
# C C O M	12	Very stiff gray clay	30/18			 -	 	+		*		++	•	-			3 EL	
224	$L \boxtimes$	-with silt partings and packets at 211' -with siltstone fragments at 223'	30/6"	-	┾═┪	 	<u> </u>	 	}_	K-			4		- +	220	*	
ē		-flocculated at 231'	30/15*	}	1		1		}}	1	8			1		1	AT10	
\ \{\bar{2}\}	LØ		30		Ţ <u></u>		<u> </u>	<u> </u>			<u>.</u>			<u>.</u>	_	240	E	
PENETRATION 54		630	1 1		L		J	<u>i</u>					_ _			١.	¥ E 1	
		Grav silt -laminated with silty clay at 251'	"		##		Ī										e E	
74	<u>'[]]]</u>	-sitty clay with shall fragments at 261*	30		7-8-7											260		
		raility sand with silt and clay packets at 271' (275'	30		 	<u> </u>	<u> </u>		 	<u> </u>		•		4		l		
20	1	Very stift gray silty clay -with silt partings at 281'	30		+ -	+	1			•	*		\top	1	*	280		
	183	-yray and dork gray or 291'	30/18"			++						≫	1	#	'			
30			30/18	-	-	<u>+</u>	┼	 	╟─	 -	00	*	-14	-[i	 	300		
	1 33		30/18"		44	! -i+]]	,				\	*	}		
32	+83	multh scottered shell fragments at 3231 medath gray fine sona layer with clayey	30/18"				 			 	-	-	~ -	<u></u>	/	320		
		tend pockets at 230" -pas in formation at 330" (330, 5)	30/12-			T	<u> </u>			↓_			<u> </u>	I	5.79	Ì		
34	, M				 				11_	<u> </u>				J		340		
		[0]			1	ì	İ	1		}			ŀ		}	l		
94	LI i	"Number of blows of a 175-lb weight dropped upproximately 5 ft required			İ		1		ll		}		1					
34	[]	to produce a 24-in, penetration, except as noted, at a 2-1/8-in, 1D, 2-1/4-in, OD sampler,				1	1						1			360		
		100, 4-10 4-in. OU tompity,	1 1						11				-			ł		
30	'				-	┼─	_	 	11-	†	1	-	+	+-	-	300		
					{				[[
40	┡┷┵			}	ــــــــــــــــــــــــــــــــــــــ	ــــــــــــــــــــــــــــــــــــــ	1161	ND FCR 3	LL	14646	TH PLO	ĻĻ		ــــــــــــــــــــــــــــــــــــــ		400		
	6	emplated at 330,5-ft panetralian (El =430,5)		• •	pentined Undergree	Gomb. es	616.0	4 1	07 7 00 8	146166	-Undreini	d Tribe	lei	İ			
					<u>ф 1</u>	19512061	**** (0	des simps		10 70	-0101	10070]				ı		

sheet 2.57 of 78_

By C. Cherr client I.S. NAUY __ subject Foundation Analysis ______ Date 6=3-76 Job No. 27-771-97 _ calculation Ripe Rile Capacity Curves _

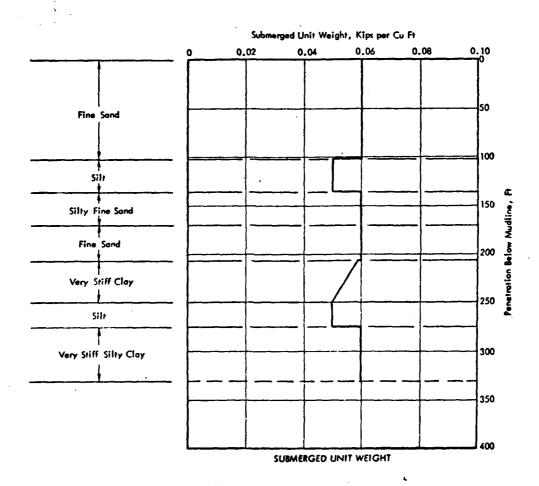
Boring #4



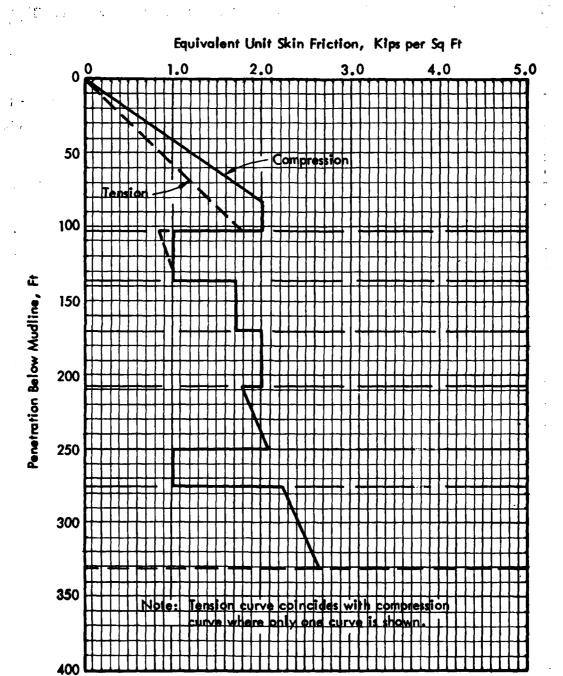
Sheet 2.58 of 78_

By C. Chern Client U.S. NAUY __ subject Foundation Analysis ____ Date 6=3-26 Job No. 27-771-97 _ calculation Pipe Pile Capacity Carries ____

Boring #4



By C. Chern client U. S. NAVY __ subject Foundation Analysis _____
Date 6=1=76 Job No. 27-77L-97_ calculation Ripe Pile Capacity Curves ___

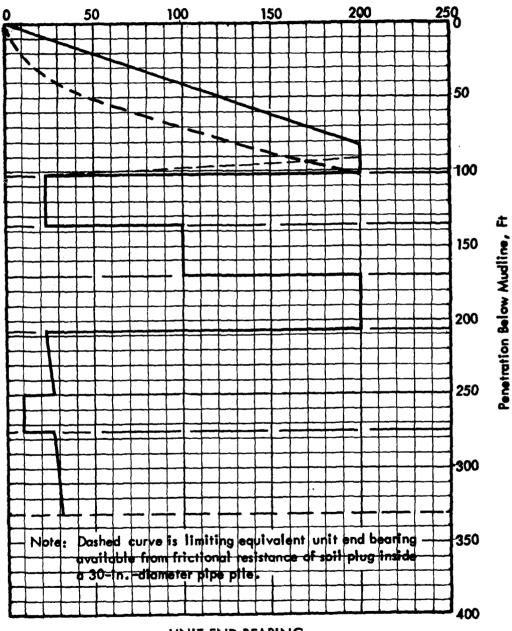


EQUIVALENT UNIT SKIN FRICTION

Boring#4

By C. Chern client U.S. NAVY __ subject Foundation Analysis _____ Date 5-1-76 Job No. 27-77/-97 _ calculation Pipe Pile Capacity Curves ___

Unit End Bearing, Kips per Sq Ft



UNIT END BEARING

Sheet 2.62 of 78__

By C. Chern client U.S. NAUY __ subject Foundation Analysis ____ Date 6=2=Z6 Job No. 27-771-97_ calculation Pipe Pill Capacity Curves __

Boring #4

PENETRATION BELOW MUDLINE	UNIT SKIN FRICTION	Unit END BEARING	SEGMENT LENGTH		
FT	KSF	KSF	PT		
25	o.40	: 	25		
25 50	0.85		25		
50 82	1.44	_ 32			
8Z 10Z	1.85	5			
102	0.85 1.00		26		
128	1.00	_	8		
170	1.70 1.70		34		
207	2.00 2.00	_	37		
207 250	1.80 2.10	-	43		
250 275	1.00	-	25		
275 330	2.25 2.65	_	55		

2330FT

UNIT CAPACITY IN TENSION

Sheet 2.63 of 78__

By C. Chern Client U.S. NAVY ___ Subject Foundation Brains Con Disch Capacity Corves __

SKIN FRICTION CAPACITY (Qs=fas As)

__ COMPRESSION ___

O.D. = 42"

(Boring #4)

As = 700 (al) = 10.996 (al) SQ.FT

			<u> </u>		·
PENETRATION	UNIT SKIN	Ave. Unn Skin		SKIN FRICTION	TOTALSKIN
BELOW MUDLINE	FRICTION	FRICTION (fas)	LENGTH (QL)	in Segment (Qs)	FRICTION
FT	KSF	KSF	PT	KIPS	KIPS
0 25	0.6	0.3	25	82.5	0 82.5
25 50	0.6	0.9	25	247.4	329.9
50 82	1.2 2.0	1.6	32	563.0	892.9
82 102	2.0 2.0	2,0	20	439.8	1,332.7
128	1.0	1.0	26	285.9	1,618.6
136	1,0 1.0	1.0	8	88.0	1,706.6
136	1.7	1.7	34	635.6	2,342.2
170	2.0 2.0	2.0	37	813.7	3,155.9
207 250	1.8	1.95	43	922.0	4,077.9
250 275	1.0	1.0	25	274.9	4,352.8
275 330	2.25 2.65	2,45	55	1,481.7	5,834.5
			≥330		

Sheet 2.61 of 78_

By C. Chern Client U.S. NAUY ___ subject Foundation Analysis ____ Date 6-2-76_ Job No. 27-771-97_ calculation Pipe Pile Capacity Curves ____

Boring #1

PENETRATION BELOW MUDLINE	UNIT SKIN FRICTION	UNIT END BEARING	SEGMENT LENGTH
FT	ksf	KSF	FT
0	0	0	
25	0.6	13.0	25
25 50	0.6	50.0	25
50 82.	2.0	135.0	32
82	2.D 2.O	195.0	20
102	I.O I.O	24.0	26
128	1.0 1.0	24.0	8
136	1.7	100.0	34
170 207	2.0 2.0	200.0 200.0	37
207 250	1.8	24.0 26.0	43
250 275	1.0	10.0	25
275 330	2.25 2.65	26.0 32.0	55

2330 FT

UNIT CAPACITY IN COMPRESSION

OFFSHORE, INC. **CREST**

Sheet 2.64 of 78__

By C. Chern client U.S. NAUX _ subject Foundation Analysis _ ____ Date 6-2-76 Job No. 27-771-97 _ calculation Pipe Pile Capa-ity Curves _ _

END BEARING CAPACITY (Qp= 9Ap)
BORING#4

Ap= 9.62 SQ.FT

 		
PENETRATION	UNIT END	
BELOW MUDLINE	BEARING	END BEARING
	(4)	(\mathcal{S}_{φ})
FT	KSF	KIPS
0	0	
25	13.0	125.1
25		
50	50.0	481.0
50	İ	{
82	135.0	1,298.7
82		
102	195.0	1.875.9
102	24.0	230.9
128		
128		
136	24.0	230.9
136	100.0	962.0
170	100.0	
170	200.0	1924.0
207	200.0	
207	24.0	230.9
250	26.0	250.1
250	10.0	96.2
275	10.0	
275	26.0	250.1
330	32.0	307.8

Sheet 2.65 of 78_

By C. Chern client U.S. NAUX _ subject Foundation Analysis ____ Date 6-2-76 Job No. 27-771-97 calculation Pipe Pile Capacity Curves _

PILE CAPACITY IN COMPRESSION (Boring #4)

PENETRATION BELOW MUDLINE	SKIN FRICTION	END BEARING	ULTIMATE CAPACITY	DESIGN CAPACITY
FT	KIPS	KIPS	KIPS	KIPS
0	0	0	0	0
25	8z.5	125.1	207.6	138.4
25				
50	329.9	481.0	810.9	540.6
50				
_82	892.9	1,298.7	2,191.6	1,461.
82				·
102	1.332.7	1,875.9	3,208.6	2,139,1
loz	_	230.9	1,563.6	1.042.4
128	1,618.6		1,849.5	1,233.0
128				
136	1,706.6		1,937.5	1,291.7
136		962.0	2,668.6	1,779.1
170	2,342.2		3,304.2	2,202.8
170		1,924.0	4,266.2	2,844.1
207	3, 155.9		5,079.9	3,386.6
207		230.9	3,386.8	2,257.9
250	4.077.9	250.1	4,328.0	2,885.3
250		96.2	4,174.	2,782,7
275	4.352.8		4,449.0	2,966.0
275		250.1	4,602.9	3,068.6
330	5,834.5	307.8	6,142.3	4,094.9

Sheet 2.66of 78_

By C. Chern Client U.S. NAUX ___ subject Foundation Analysis ____ Date 6-2-76 Job No. 27-171-97 _ calculation Papa Pile Capacity Curves ___

SKIN FRICTION CAPACITY (Qs = fas As)

__ TENSION _ _

O.D. = 42"

BORING#4

As = 70D (al) = 10.996 (al) SQ.FT

PENETRATION	UNIT SKIN	AVE. UNITSKIN	SEGMENT	SKIN FRICTION	TOTAL SKIN
BELOW MUDLINE	FRICTION	FRICTION (fas)	LENGTH (AL)	INSEGMENT (Qs)	FRICTION
FT	KSF	KSF	FT	KIPS	KIPS
25 0	0 0.40	0.20	25	55.0	0 55.0
50 25 50	0.40 0.85	0.62	25	170.4	225.4
50 82	0,85 1,44	1.14	32	401.1	626.5
82 102	1.44	1.64	20	360.7	987.2
102 128	0,85 1.00	0.92	26	263.0	1,250.2
128 136	1.00	1.00	8	88.0	1,338.2
136	1.70	1.70	34	635.6	1,973.8
170	2.00 2.00	2.00	37	813.7	2,787.5
207 250	1.80 2.10	1.95	43	922.0	3,709.5
250 27 5	1.00	1.00	25	274.9	3,984.4
275 330	2.25 2.65	2.45	55	1.481.7	5,466.1
			2 330		

sheet 2.67 of 78

By C. Chern Client U.S. NAUY __ subject Foundation Analysis ____ Date 6-2-76 Job No. ZZ-771-97 _ calculation Pipe Pile Capacity Curves __

PILE CAPACITY IN TENSION (Boring #4)

PENETRATION		
BELOW	ULTIMATE	DESIGN
MUDLINE	CAPACITY	CAPACITY
		(F.S.=1.5)
FT	KIPS	KIP3
25	0	0
25	55.0	36.7
25		
50	225.4	150.3
50		
82	626.5	417.7
82		
102	987.2	658.1
102		
128	1.250.2	833.5
128		
136	1.338.2	892.1
136		
170	1.973.8	1.315.9
170		
207	2,787.5	1,858.3
207		
250	3.709.5	2,473.0
250		
275	3,984.4	2,656.3
275		
330	5,466.1	3,644.

Sheet 2.68 of 78__

By C. Chern client LI.S. NAVY __ subject Foundation Analysis _____
Date 6=3-26 Job No. 27=771=97 calculation Experile Capacity Curves ___

Modification on Unit End Bearing Capacity

Boring #4

End Bearing

$$Q_p = (170 \text{ KSF}) \times (9.62) = 1,635.4 \text{ Kips}$$

Skin Friction

$$Q_s = 892.9 + 439.8 \times \frac{91.5 - \beta 2}{20} = 1,101.8 \text{ kips}$$

Ultimate Capacity

Design Capacity

(ii) At Penetration 146.5 FT (=136'+10.5')

End Bearing

$$Q_p = (100 \text{ KSF}) \times (9.62) = 962 \text{ Kips}$$

Skin Friction

$$Q_s = 1.706.6 + 635.6 \times \frac{10.5}{34} = 1,902.9$$

Ultimate Capacity

$$Q = 96z + 1,90z.9 = 2,864.9$$
 KIPS

Design Capacity

(iii) At Penetration 180.5 FT (= 170'+10.5')

End Bearing

$$Q_{\rho} = (200 \text{ KSF}) \times (9.62) = 1,924 \text{ KIPS}$$

Skin Friction

$$Q_s = 2.342.2 + 813.7 \times \frac{10.5}{37} = 2.573.1^{kips}$$

Ultimate Capacity

$$Q = 1,924 + 2,573.1 = 4,497.1 \times 1PS$$

Design Capacity

Sheet 2.69 of 78_

By C. Chein Client U. S. NAVY __ subject Foundation Analysis _____
Date 6-3-26 Job No. 27-771-97 calculation RipePile Capacity Curves ____
BORING #4

(1v) At Penetration 196.5 FT (=207-10.5)

End Bearing

Qp = (200 KSF) x (9.62) = 1,924 KIPS

Skin Friction

 $Q_s = 2,342.2 + 813.7 \times \frac{196.5 - 170}{37} = 2,925^{kips}$

Ultimate Capacity Q=1,924+2,925=4,849 Kirs

Design Capacity Qd = 3,232.7 KIPS

(V) At Penetration 239.5 FT (= 250-10.5)

End Bearing

 $Q_p = (25.5 \text{ KSF}) \times (9.62) = 245.3 \times 195$

Skin Fiction

 $Q_s = 3,155.9 + 922 \times \frac{239.6 - 207}{43} = 3,852.8 \text{ kips}$

Ultimate Capacity

Q = 246.3 + 3,852.8 = 4,098.1 KIPS

Design Capacity

Qd = 2,732. | Kips

(Vi) At Penetration 285.5 FT (=275'+10.5')

End Bearing

 $\Theta_p = (27 \text{ KSF}) \times (9.6z) = 259.7 \text{ KIPS}$

Skin Friction

 $Q_s = 4.352.8 + 1.481.7 \times \frac{10.5}{55} = 4.635.7$

Ultimate Capacity

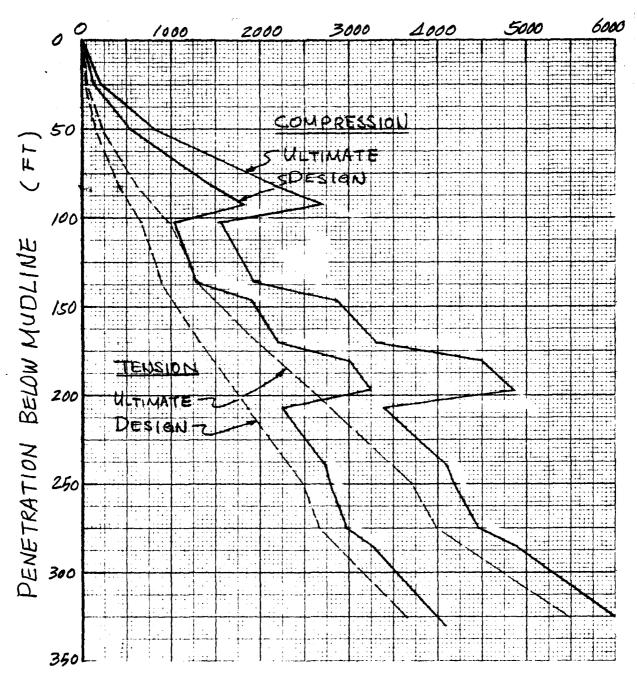
Q = 259.7+4,635.7=4,895.4 KIPS

Design Capacity

Qd= 3,263.6 KIPS

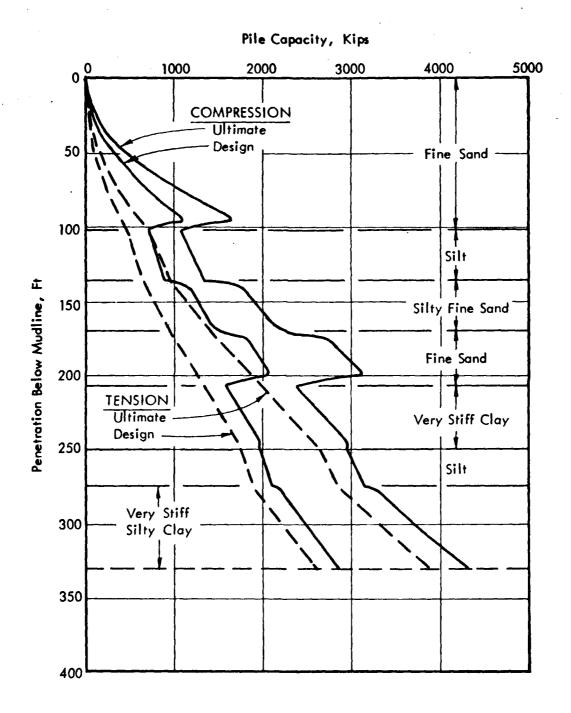
By C. Chern client U.S. NAVY __ subject Foundation Analysis ____ Date 6-3=26 Job No. 27-771-91 calculation Pipe Pile Capacity Curves __

PILE CAPACITY (KIPS)



42-IN. DIAMETER PIPE PILES (Boring #4)

By C. Chern client U.S. NAVY __ subject Foundation Analysis ____
Date 6-3-76_ Job No. 27-771-97 _ calculation Ripe Life Capacity Curves ___



PILE CAPACITY CURVES 30-in. Diameter Pipe Piles Boring 4

2.6 CAPACITY CURVES FOR BORING SITE NO. 4
WITH 33" DIAMETER INSERTED PILING

By C. Chern client U.S. NAVY _ subject Foundation Analysis ____ Date 8-25-76 Job No. 27-771-97 _ calculation Ripe Pile Capacity Curves _

SKIN FRICTION CAPACITY (Qs=fasAs)
$$O.D.=33"$$

CONIPRESSION — O.D. = 42" $As' = \pi D_3(Al)$

= 8.639(AL) SQ.PT

(Boring #4)

As = 700(al) = 10.996 (Al) SO.FT

. 0			T-		
PENETRATION BELOW MUDLINE	1	Ave. (lum Skin Friction (fas)	SEGMENT LENGTH (al)	SKIN FRICTION IN SEGMENT (Qs)	TOTALSKIN . FRICTION
FT	KSF	KST	FI	KIPS	KIPS
25	0.6	0.3	25	82.5	0 82.5
25 50	0.6	0.9	25	247.4	329.9
50 82	1.2	1.6	32	563.0	892.9
82 102	2.0 2.0	2,0	20	439.8	1.332.7
102.	1.0	1.0	26	285.9	1.618.6
128	1.0	1.0	8	88.0	1,706.6
136	1.7	1.7	34	635.6	2,342.2
170	2.0	2.0	37	813.7	3,155.9
201	1.8	1.95	43	724.4*	3,880.3 4,077.9
250 275	1.0	1.0	25	216.0 * 274.9	4,096.3
275 330	2.25	2.45	55	1,164.1*	5,260.4 5,834.5
			≤ 330		

^{*} denotes the contribution from 33" of inserted piling.

Sheet 2014 of 78__

By C. Chern- client U.S. NAVY _ subject Foundation Anolysis ______ Date 8-25-76 Job No. 27-771-97 _ calculation Pipe Pile Capacity Curves _

END BEARING CAPACITY Qp = & Ap

BORING #4

		
PENE TRAITION	UNIT END	•
BELOW MUDLINE	BEAKING	END BEARING
	(7)	(Q_{φ})
FT	KSF	KIPS
0	0	
25	13.0	125.1
び		
50	50.0	4.81.0
50		
82	135.0	1,298.7
82		
102	195.0	1.875.9
102.	24.0	230.9
128		
128		
136	24.0	230.9
136	100.0	962.0
170	100.0	
170	200.0	1924.0
201	200.0	
201	24.0	142.6*
250	26.0	154.4*
25,0	10.0	59.4*
275	10.0	
275	26.0	154.4*
330	32.0	190.1*

 $A_p = 9.62$ sa.FT for $42''\phi$ piling $A_p = 5.94$ sa.FT

for 33" piling

* denotes the contribution from 33" & inserted piling

Sheet 2.75 of 78__

By C. Chern client U.S. NAVY _ subject Foundation Analysis _____ Dute 8-25-76 Job No. 27-771-97 _ calculation Pipe Pile Capacity Curves _

PILE CAPACITY IN COMPRESSION (Boring #4 -- 33 4 Inserted Piling)

PENETRATION BELOW ALIDEINE	SKIN FRICTION	END BEARING	ULTIMATE CAPACITY	DESIGN CAPACITY
FT	KIPS	KIPS	KIPS	KIPS
0	0	0	0	U
25	82.5	125.1	207.6	138.4-
25 50	329.9	481.0	810.9	1,40.6
50 82	892.9	1,298.7	2,191.6	1,461.
82. 102	1.332.7	1,875.9	3,208.6	2,139.1
201		230.9	1,563.6	1.042.4
128	1,615.6		1,849.5	1,233.0
128	1,706.6		1,937.5	1,291.7
136		962.0	2,668.6	1,779.1
170	2,342.2		3,304.2	2,2.02.8
170		1,924.0	4,266.2	2,841.1
207	3, 155.9		5,079.9	3,386.6
207	1	142.6	3, 298.5	2,199.0
250	3,880.3	154.4	4,034.7	2,689.8
250		59.4	3,939.7	2,626.5
275	4,096.3		4,155.7	2,770.5
275		154.4	4,250.7	2,833.8
320	5,260.4	190.1	5,450.5	3,633.7

By C. Chern client U.S. NAUY __ subject Foundation Analysis ___ Date 8-25-76 Job No. 27-771-97 _ calculation Pipe Pila Capacity Curves

SKIN FRICTION CAPACITY (Qs = fas As) { O.D. = 38"

 $-T_{ENSION} - - 0.D. = 42" \begin{cases} A'_{s} = \pi D_{ss} (\Delta L) \\ = 8.639 (\Delta L) sa.FT \end{cases}$

BORING #4

(

As = 7 D_(AL) = 10.996(AL) SQ.FT

0		A 41. C.		C. T	
PENETRATION		AVE. UNITSKIN		SKIN FRICTION	}
BELOW MUDLINE	FRICTION	FRICTION (fas)	LENGTH	INSEGMENT	FRICTION
FT	KSF	KSF	(<u>A</u> l) FT	KIPS	KIPS
0	0				٥
25	0.40	0.20	25	55.0	55.O
25	0.40	0.60	25	1 0 . 4	
50	0.85	0.62	25	170.4	225.4
50	0.85	1.14	32	1011	
82	1.44	1.1-1	7.5	401.1	626.5
82	1.41	1.64	20	360.7	
102	1.85	(, 0 - 1		300.7	987.2
102	0.25	0.92	26	263.0	
128	1.00				1,253.2
128	1.00	1.00	8	88.0	1 2 5 2 5
136	1.00	·			1.338.2
136	1.70	1.70	34	635,6	10720
170	1.70			<u> </u>	1,973.8
207	2.00 2.00	2.00	37	813.7	2.787.5
207	1.80			724.4*	3,511.9
250	7.10	1.95	43	724.4*	3,7,7,4
250	1.00		0.2	216.0*	3,727.9
275	1.00	1,00	25	274.9	20214
275	2.25	C-1C	55	1,164.1*	4,892.0
330	2.65	2.45	27	1.481.7	5-406-1
			2330		
				1	

* denotes the contribution from 33" inserted piling

Sheet 2.77 of 78__

By C. Chern Client U. S. NAVY _ subject Foundation Analysis ____ Date B-25-76 Job No. 27-771-97 _ calculation Pipe Pile Capacity Curves _

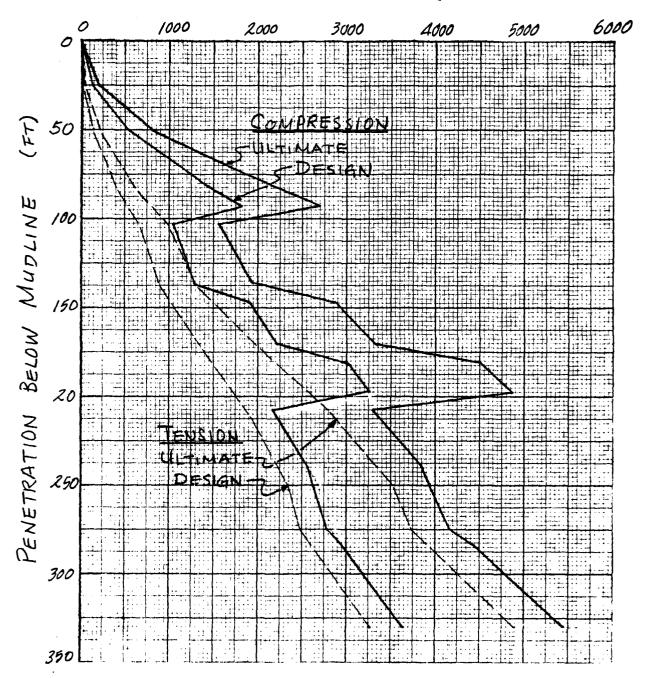
PILE CAPACITY IN TENSION (Boring #4)

PENETRATION		
BELOW	ULTIMATE.	DESIGN "
MUDLINE:	CAPACITY	CAPACITY
		(F.S.=1.5)
FT	KIPS .	KIPS
. 0	0	0
25	55.0	36.7
25		
50	225.4	150.3
50		
82	626.5	4.17.7
82		
102	987.2	678.
102		
128	1.250.2	833.5
128		
136	1.338.2	892.1
136		
170	1,973.8	1.315.9
170		
207	2,787.5	1,858.3
201	3,511.9	2,341.3
250	3.709.5	2,473.0
250	3,727.9	2,485.3
2:15	3,954.4	2,4,6,3
275	4,892.0	3,261.3
330	6.466.	3.614
		·

()

Sheet 2.78 of 78

PILE CAPACITY (KIPS)



42-IN. DIAMETER PIPE PILES
(200 FT PENETRATION)
W/ 33-IN DIAMETER INSERTED PILES
(Boying #4)

SECTION 3

PILE DRIVING RESISTANCE CURVES

3.1 INTRODUCTION

Driving resistance curves are developed for 42-inch diameter pipe piles at boring site Nos. 1, 2, 3A and 4, respectively. The method used in this section is the stress-wave approach as presented in the McClelland Report. It should be noted that these curves are empirical and approximate in nature and in no way assure attainment of the desired penetration.

3.2 ESTIMATED DRIVING RESISTANCE CURVES

OFFSHORE, INC. **CREST**

Sheet 3:03 of 66_

By C. Chern client U.S. NAVY __ subject Foundation Avalysis _____
Date 6=11-16 Job No. 27-771=91 _ calculation Bile Driving Resistance Crues

Skin Friction Capacity (Qs = fas As) *50% SKIN FRICTION IN CLAY

--- Compression --- 0.D. = 42"

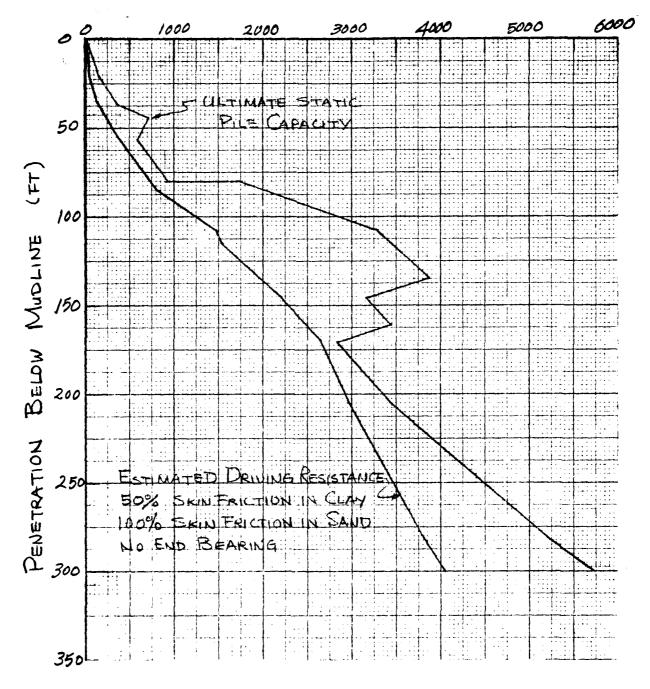
(Boring #1)

As = 70 D(al) = 10,996 (al) SQ.FT

Pencylotion	Unid Skin	Ave. Chilskin	Segment	SkinFriction	Total
Below Midling	Friction	Friction	Leigth	in Segment	skinFrictim
(ft)	(ksf)	fas (KSF)		(Kipe)	(kips)
0	0	0 7 7 5		105	O
20	0.45	0.225	20	49.5	49.5
20	0.40	0.535	15	88.2	/37.7
35 56	0.85	1.075	21	248.2	385.9
56 79	1.10	1.375	2.3	347.7	733.6
79	2.00	1.950	3	64.3	797.9
82	2.00	2.00	25	549.8	1,347.7
107	2.00	2.00	8	175.9	1.523.6
115	2.00 2.00	2.00	30	657.8	2.1834
145	1.70	1.70	25	467.3	2,650.7
170	1.48	1.60	35	307.9* 615.8	z,958.6 3.266.5
205	2.30	2.0(75	828.8*	3,787.4 4-724.1
280	2.30	2.35	20	258.4* 516.8	4,045,8 5,440,9
		·	Z= 300 ft		

By C. Chrim client U.S. NAUX __ subject Foundation Analysis ____ Date 6-11-76 Job No. 27-771-97 _ calculation Pile Driving Resistance Curves

ULTIMATE STATIC PILE CAPACITY (KIPS) ESTIMATED DRIVING RESISTANCE (KIPS)



(Boring #1)
42-IN. DIAMETER PIPE PILES

By C. Chern client U.S. NAVY _ subject Foundation Analysis _____
Date 6-4-76 Job No. 27-771-91 _ calculation Pile Driving Resistance Curves

SKIN FRICTION CAPACITY (Q= fas As)

#50% Skin Friction IN CLAY

-- COMPRESSION --

O.D. = 42"

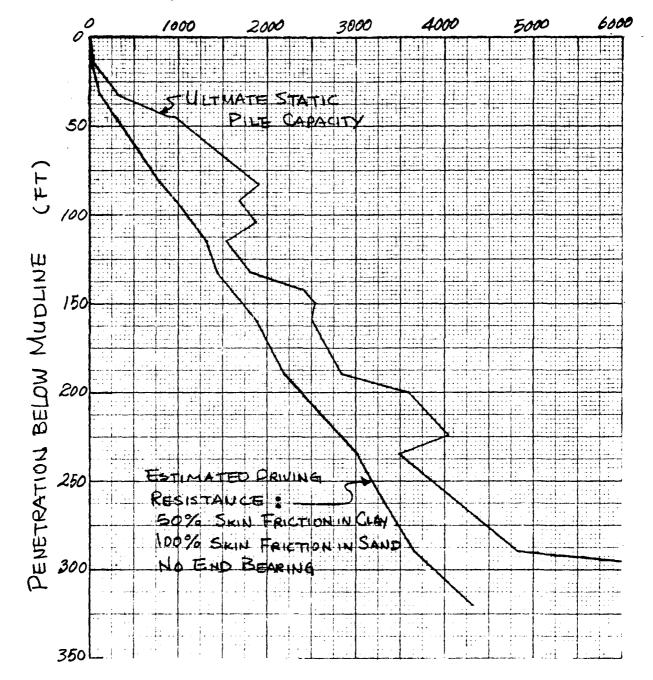
(Boring #2)

As = MD(AL) = 10.996(AL) SQ.FT

PENETRATION	UNIT SKIN	AVE. UNITSKIN	SEGMENT	SkIN FRICTION	TOTAL SKIN
BELOW MUDLINE	FRICTION	FRICTION (fas)	LENGTH (AL)	IN SEGMENT	FRICTION
FT	KSF	KSF	FT	KIPS	KIPS
15	0.30	0.15	15	24.7	24.7
15 33	0,30	0.50	18	99.0	123.7
33 40	2.15	2.21	7	85.0* 170. 	208.7 293.8
40	2.27	2.33	5	64.0* 128. 1	272.7 421.9
45 82	0.90	1.30	37	528.9	801.6 950.8
8Z 91	1.70	1.70	9	168.2	969.8 +.++9.0
91	1.40	1.40	23	354.	1,323.9
132	0.90 0.90	0,90	18	89.0 * +78.+	1,412.9 +,651.2
132 160	1.40	1.40	28	431.0	1,843.9 2,082.2
160	1.00	1.00	30	329.9	2,17 3. 8 2,412.1
190 235	1.62	1.66	45	821.4	2,995.2 3,233.5
235	1.80	2.00	35	384.9* 769.7	3,380.1 4,00 3.2
270 290	2.20	2.27	20	249.6* 499.2	3,629.7 4.502.4
290 320	2.00	2.00	30	6 59.8	4,289.5 5,162.E

By C. Cherry Client U.S. NAVY ___ subject Foundation Analysis _____ Date 6-4-76 Job No. 27-77/-97 _ calculation Pile Driving Resistance Conves

ULTIMATE STATIC PILE CAPACITY (KIPS)
ESTIMATED DRIVING RESISTANCE (KIPS)



(Boring #2)
42-IN DIAMETER PIPE PILES

By C. Chern client U.S. NAUY ___ subject Foundation Analysis _____
Date 6-4-76 Job No. 27-77L-91 _ calculation Pide Driving Resistance Curves

SKIN FRICTION CAPACITY (Qs = fas As) *50% SKIN FRICTION

IN CLAY

-- COMPRESSION --

(Boring #3A)

0.D. = $4z^*$ $A_s = \pi D(\Delta l) = 10.996(\Delta l) \text{ SQ.FT}$

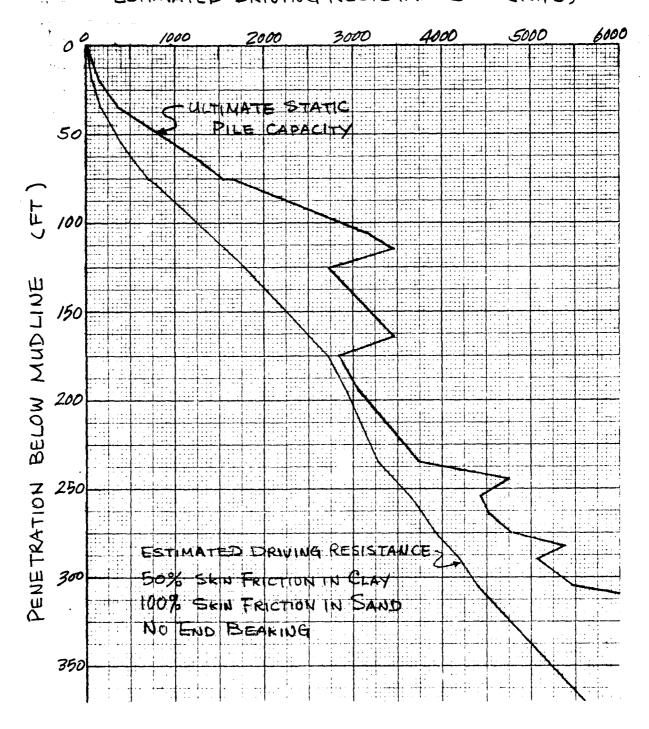
PENETRATION	UNIT SKIN	AVE LAUTSKIN	SEGMENT	SKIN FRICTION	TOTAL SKIN
BELOW MUDLINE		FRICTION	LENGTH	IN SEGMENT	FRICTION
	TRICTION	(fas)	(Al)	(Qs)	TRICTION
FT	KSF	KSF	FT	KIPS	KIPS
20	0. 0.5	0.25	20	55.0	55.0
35 20	0.5	0.67	15	110.5	165.5
35 55	0.85	1.07	70	235.3	400.8
65 55	1.10	1.20	10	131.9	532.7
75 65 75	1.30	1,40	10	153.9	686.6
82	2.00	1.90	7	146.2	832.8
106	2.00	2.00	24	527.8	1,360.6
106	2.00	2.00	9	197.9	1,558.5
115	2.00 2.00	2.00	10	219.9	1,7784
175	1.70	1.7	50	934.7	2,7/3.1
175	1.00	1.0	20	219.9	2,933,0
195 235	1.53	1.55	40	340.9 * 681.8	3,273.9 3,614.8
235 265	1.70	1.70	20	373.9	3,647.8 3,988.7
265	2.63 2.63	2.63	10	144.6* 289.2	3,792.4
265 275	2,20	2.20	10	120.9*	3,913.3 4,519.8
275	1.70	1,70	15	280.4	4,193.7 4,800.2
290	2,45 2.40	2.42	15	199.6* 399.2	4,393.3 5.199.4
305 370	1.70	1.70	65	1,215.	5,608.4 6,414.5
			€370		

Sheet 3.03 of 66

By C. Chern. Client U.S. NAVY ___ subject Foundation Analysis_____
Date 6-4-76 Job No. 27-771-97 _ calculation Rile Driving Resistance Curves_

ULTIMATE STATIC PILE CAPACITY (KIPS)

ESTIMATED DRIVING RESISTANCE (KIPS)



(Boring #3A)
42-IN. DIAMETER PIPE PILES

By C. Chern Client U.S. NAVY _ subject Foundation Projects _____ Date 6-4-76 Job No. 27-77L-91 _ calculation Fele Driving Resistance Curves

SKIN FRICTION CAPACITY (Qs=fas As) *50% SKIN FRICTION IN CLAY

-- CONIPRESSION ---

O.D. = 42"

(Berling #4)

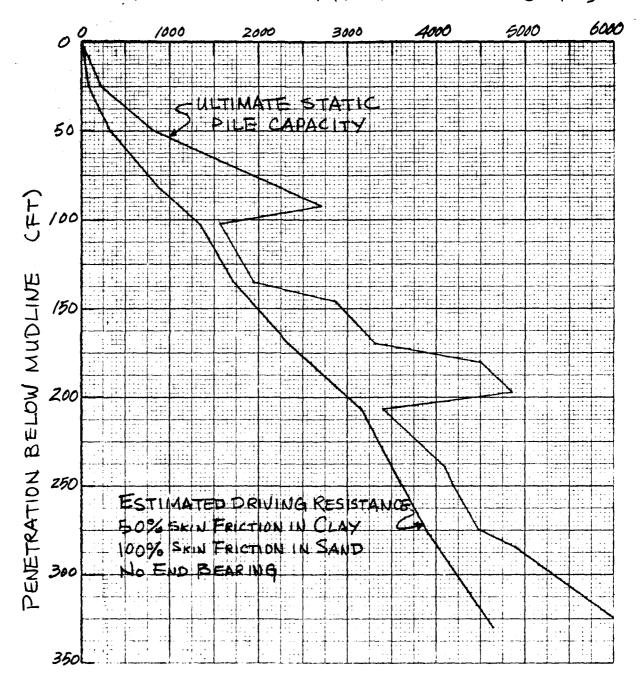
As = 7(D(al) = 10.996 (Al) SQ.FT

PENETRATION	UNIT SKIN			SKIN FRICTION	
BELOW MUDLINE	FRICTION	FRICTION (fas)	LENGTH (QL)	IN SEGMENT	FRICTION
FT	KSF	KST	FT	KIPS	KIPS
0 25	0.6	0.3	25	82.5	<i>C</i> 82.5
50 50	0.6	0.9	25	247.4	329.9
50 82	1.2 2.0	1.6	32	563.0	892.9
10%	2.0	2,0	20	439.8	1,332.7
102	1.0	1.0	26	285.9	1.618.6
136	1.0 1.0	1.0	8	88.0	1,706.6
170	1.7	1.7	34	635.6	2,342.2
170	2.0 2.0	2.0	37	813.7	3,155.9
207	1.8	1.95	43	461.0*	3,616.9 4,077.9
250 275	1.0	1.0	25	274.9	3,891.8 4,352.8
275 330	2.25 2.69	2,45	55	740.8*	4.632.6 5,834.5
			≤330		

Sheet 3:10 of 66

By C. Chern Client U.S. NAUY __ subject Foundation Analysis ____ Date 6=4-76 Job No. 27-771-97 _ calculation Pile Driving Resistance Curves

ULTIMATE STATIC PILE CAPACITY (KIPS)
ESTIMATED DRIVING RESISTANCE (KIPS)



(Boring #4)
42-IN. DIAMETER PIPE PILES

FOUNDATION ANALYSIS ERST COAST AIR COMBAT MANEUVERING RANGE OFFSHORE KITT. (U) CREST ENGINEERING INC TULSA OK SEP 76 27-771-97 CHES/NAVFAC-FPO-7612 N62477-76-C-0179 F/G 13/13 AD-A163 522 2/6 UNCLASSIFIED NL



MICROCOPY RESOLUTION TEST CHART

Sheet 3:11 of 66__

By C. Chern client U.S. NAVY _ subject Foundation: Analysis _____
Date B = 25 = 76 Job No. 27 - 77 L-97 _ calculation Pipe Pile Capacity Curves

SKIN FRICTION CAPACITY (Qs=fas As)

33" Inserted piling

-- COMPRESSION --

O.D. = 42"

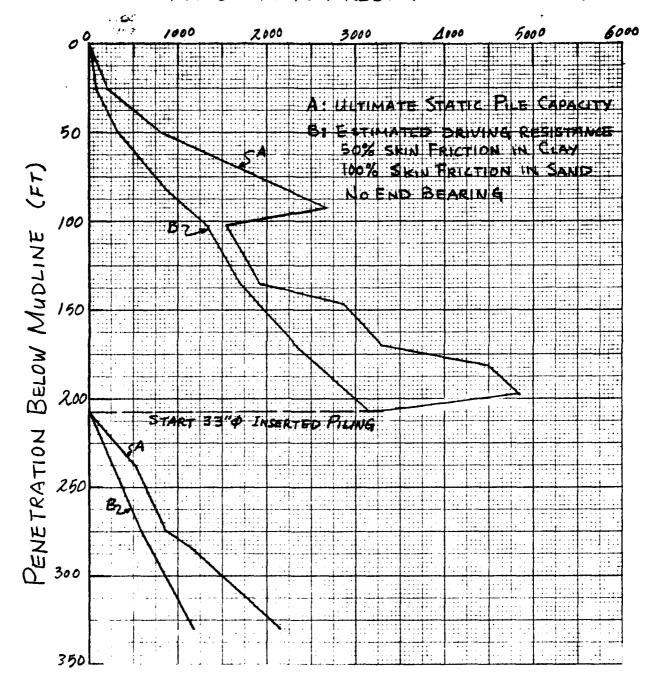
(Boring #4)

As = 70 (al) = 10.996 (al) SQ.FT

PENETRATION	UNIT SKIN	Ave. Unit Skin	SEGMENT	SKIN FRICTION	TOTALSKIN
BELOW MUDLINE		FRICTION (fas)	LENGTH	IN SEGMENT	FRICTION
FT	KSF	KST	(al) FT	(Q.) KIPS	KIPS
25 25	0.6	0.3	25	82.5	0 82.5
25 50	0.6	0.9	25	247.4	329.9
50 82	1.2 2.0	1.6	32	563.0	892.9
82 102	2.0 2.0	2,0	20	439.8	1,332.7
102	1.0	1.0	26	285.9	1.618.6
128 136	1.0 1.0	1.0	8	88.0	1.706.6
136 170	1.7	1.7	34	635.6	2,342.2
170 207	2.0 2.0	2.0	37	813.7	3,155.9
207 250	1.8	1.95	43	362.2* 922.0	362.2 4.077.9
250 275	1.0	1.0	25	216.0 274.9	578.2 4.352.8
275 330	2.25 2.65	2.45	55	582.0* +,481-7	1,160.2 5,834.5
			≤330		

DOTO 8-26-76 JOB NO. 27-771-97 - COLCULATION Pipe Pile Capacity Curves
ULTIMATE STATIC PILE CAPACITY (KIPS)

ESTIMATED DRIVING RESISTANCE (KIPS)



42-IN. DIAMETER PIPE PILES (200 FT PENETRATION)

33-IN. DIAMETER INSERTED PILES

(Boring #4)

3.3 PILE SCHEDULE NO. 1 -- 1 IN.
MINIMUM WALL THICKNESS

By C. Chexn. Client U.S. NAUX _ subject Foundation Analysis ____ Date 6-2-76 Job No. 27-771-91 _ calculation Pile Driving 12: If and Carves

	· · · · · · · · · · · · · · · · · · ·						,	
,	30-0		•	1.25 5	p-6	•		
. 0-,091	50,00		3	1.50 WT	P- 5			
91 .	50.0"		•	1.50 WT	P-4			
	50-0"	20,06 ,0,02	~1/*/ <i>[</i>	N 1.25 WT	P-3	757	<u>~/</u> ~/	-
150,-0"	50-0"			1.25 W	P-2			
150	80:00		1	1,00 wT	1-0			

MLW = 105'-0"

150 FT Penetration

Vulcan 560 Hammer
Wt. of Ram = 60,000 lbs
Rated Energy = 300,000 ft-lbs
Hammer Efficiency = 0.75
Wt. of Pile Cap = 42,000 lbs

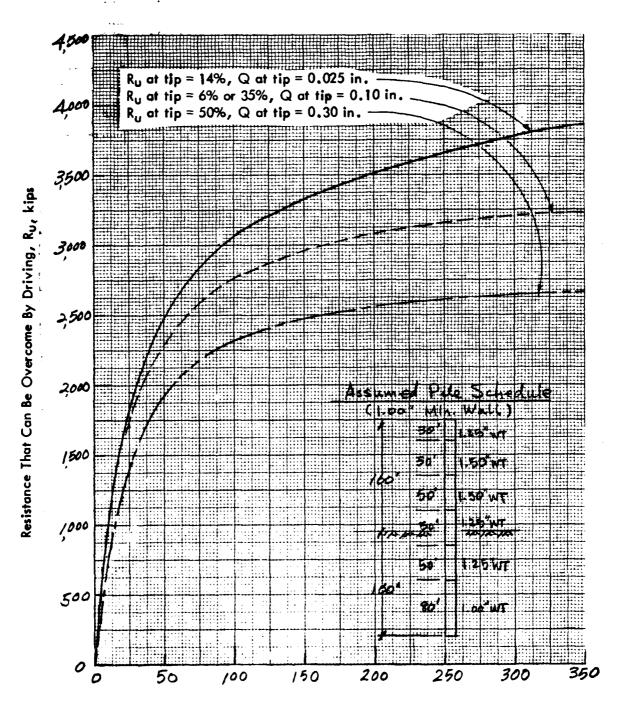
Spring Constant = 6.2 x 10⁶ lbs/in.

Damping Factor, side & tip, J = 0.15

Quake Factor, side, Q = 0.10

Quake Factor, tip, - See Above

By C. Cherr client U.S. NAVY __ subject Foundation Analysis ____ Date 6-9-76 Job No. 27-771-97 _ calculation Pile Driving Resultance Curves



Rate of Penetration, N, Blows per Foot 150-Ft Penetration

By C. Chern Client II. S. NAVX _ subject Foundation Analysis _ ___ Date 6-2-76 Job No. 27-771-91 _ calculation Pile Driving Resistance Cornes

	, 0-,002			160	,0-,091	1
80.0	50'-0"	20,0%	1 .0,05	50.0"	.0-05	30,00
			20,02 30,00	\	,	
			**************************************	- <u>-</u> -		
F3".00.	 Tw'00.+	1.25 "WT	- TW.25.1%	1.50 "WT	1.50"WT	1.25"
	P-2	P-3	P-4	P-5	P-6	P-7
			<i>T</i>			
			<i>TP-1</i>			:
			(×1)			:
			\$/,			•
			Ş			

MLW = 105'-0"

200 FT Penetration

Vulcan 560 Hammer
Wt. of Ram = 60,000 lbs
Rated Energy = 300,000 ft-lbs
Hammer Efficiency = 0.75
Wt. of Pile Cap = 42,000 lbs

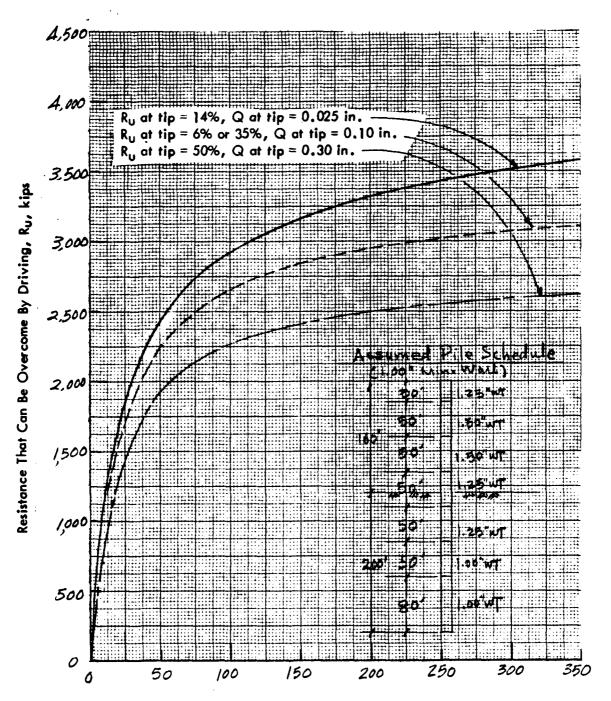
Spring Constant = 6.2 x 10⁶ lbs/in.

Damping Factor, side & tip, J = 0.15

Quake Factor, side, Q = 0.10

Quake Factor, tip, - See Above

By C. Charn client U.S. MAUX __ subject Foundation Analysis ____ Date \$2-9-76_ Job No. 27-771-97 _ calculation Pile Driving Residence Curses_

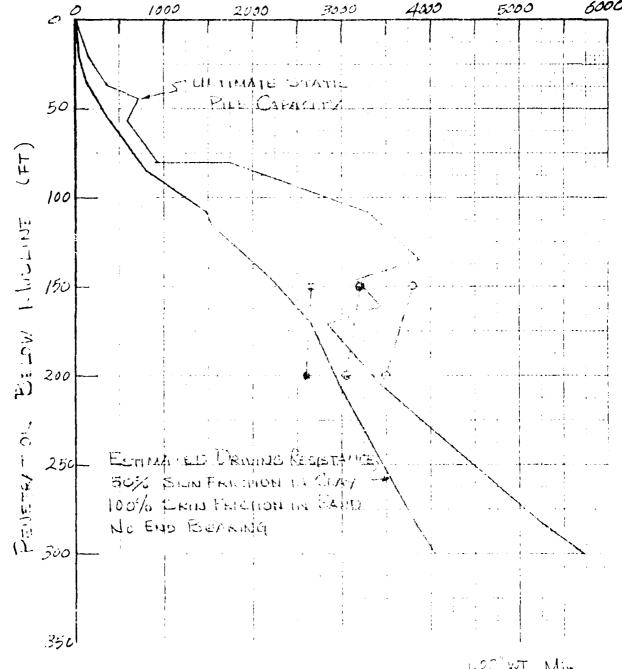


Rate of Penetration, N, Blows per Foot 200 -Ft Penetration

42-IN DIAMETER PIPE PILES

By C. China Client U. MAUX Subject Foundation Analysis Date G=11-16 Job No =1 12 21 97 _ Catculation Tale Language Ke alle Colores ULTIMATE STATIC PILE CAPACITY (KIPS)

ESTIMATED DRIVING RESISTANCE (KIPS)



Valor 560 Hammer

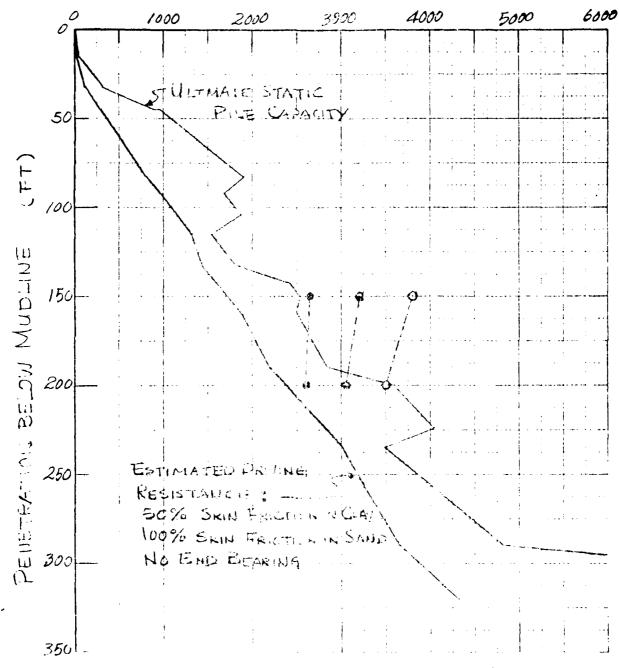
(Boring # 1)

42-11 DIAMETER FILES

Date 6-4-26 Job No. 27-771-97 _ calculation Pile Driving Resistance Curves

ULTIMATE STATIC PILE CAPACITY (KIPS)

ESTIMATED DRIVING RESISTANCE (KIPS)



100"WT Min.

Vulcan 560 H

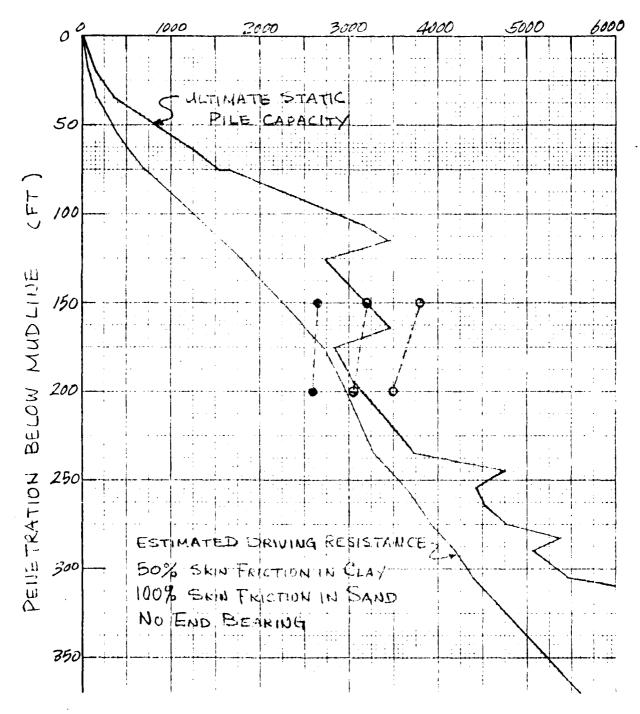
(Kerny #2)

42-IN DAMETER PIPE PILES

Sheet 3.20 of 66_

By C. Chern Client U.S. NAVY _ subject Fourilt tion Analysis ____ Date 6=4-76 Job No. 27-771-91 _ caiculation Pile Driving Recistance Curves

ULTIMATE STATIC PILE CAPACITY (KIPS)
ESTIMATED DRIVING RESISTANCE (KIPS)



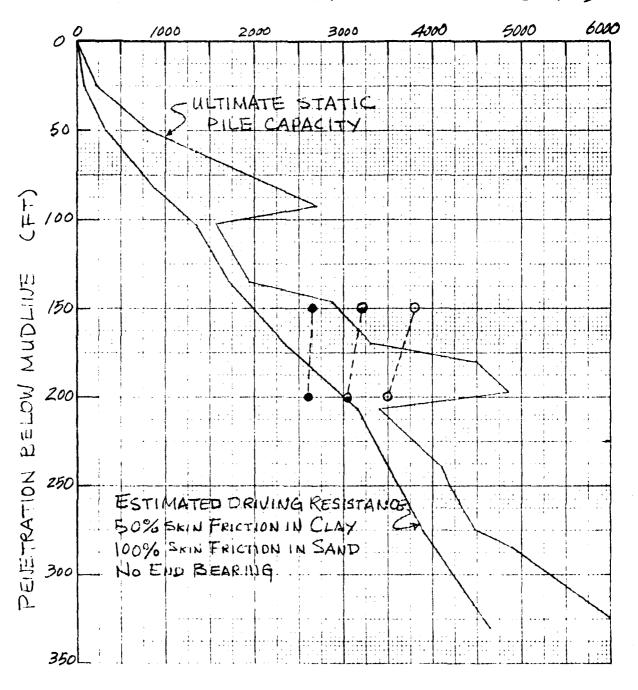
1.00 WT Min.

(Boring #3A)
42-IN. DIAME OR PIPE PILES

Vulcon 560 Hammer

Sheet 3.21 of 66_

ULTIMATE STATIC PILE CAPACITY (KIPS)
ESTIMATED DRIVING RESISTANCE (KIPS)



1.00"WT Min. Vulcan 560 Hammer

(Boring #4)

42-III. DIAMETER PIPE PILES

3.4 PILE SCHEDULE NO. 2 -- 1.25 IN.
MINIMUM WALL THICKNESS

By C. Chern client U.S. NAVY _ subject Franchation Againsi Date 6-8-76 Job No. 27-771-97 calculation Pulk Driving Residence Cornes

	30-0"				1.50 WT	9-d	
160,-0"	50,00				TW" CT.1		
	50.0"		•		T3"21.	م م	
+	50,0.	20,00, 30,00	151	Z~7,	1.50 WT	D-3	TETE TETES
150'-0"	50,-0"				1.50 "WT	2-0	
	0,00				1.25",47	0-	·

MLW = 105'-0"

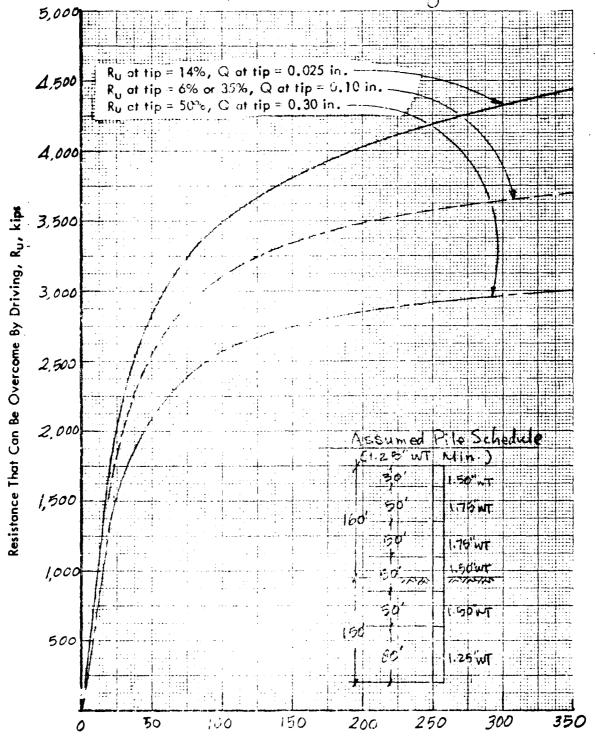
150 FT Penetration

Vulcan 560 Hammer
Wt. of Ram = 60,000 lbs
Rated Energy = 300,000 ft-lbs
Hammer Efficiency = 0.75
Wt. of Pile Cap = 42,000 lbs

Spring Constant = 6.2 x 10⁶ lbs/in.
Damping Factor, side & tip, J = 0.15
Quake Factor, side, Q = 0.10
Quake Factor, tip, - See Above

By C. Chorin client 11 S. NAUX subject Foundation Analysis

Date 6-10-76 Job No. 27-771-9.7 colouistion Pile Driving Resistance Curves

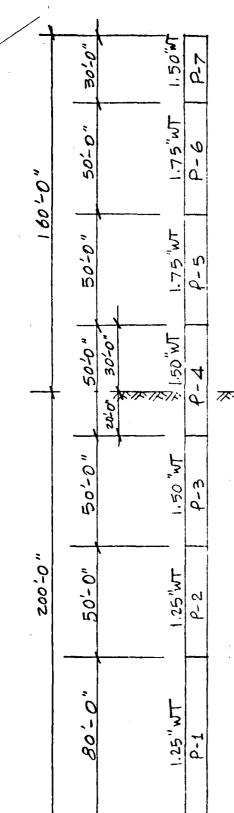


Rate of Penetration, N. Blows per Foot 150 -Ft Fanatration

42-IN. DAMETER PIPE PILES

Sheet = 5 of 66

By C. Chern Client U.S. NAVY _ subject Foundation Analysis _____ Date 6-8-76 Job No. 27-771-97 _ calculation Pile Driving Resistance Curves



MLW = 105'-0"

200 FT Penetration

Vulcan 560 Hammer
Wt. of Ram = 60,000 lbs
Rated Energy = 300,000 ft-lbs
Hammer Efficiency = 0.75
Wt. of Pile Cap = 42,000 lbs

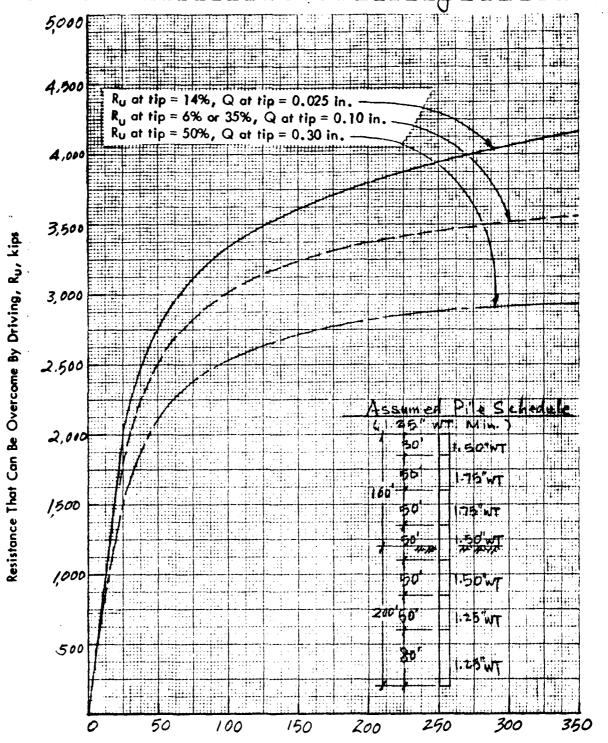
Spring Constant = 6.2 x 10⁶ lbs/in.

Damping Factor, side & tip, J = 0.15

Quake Factor, side, Q = 0.10

Quake Factor, tip, - See Above

By C. Chern Client U.S. NAUY ___ subject Foundation Augustic Augustic Date 6=10-76 Job No. 27-771-97_ calculation Pile Driving Resistance Craves

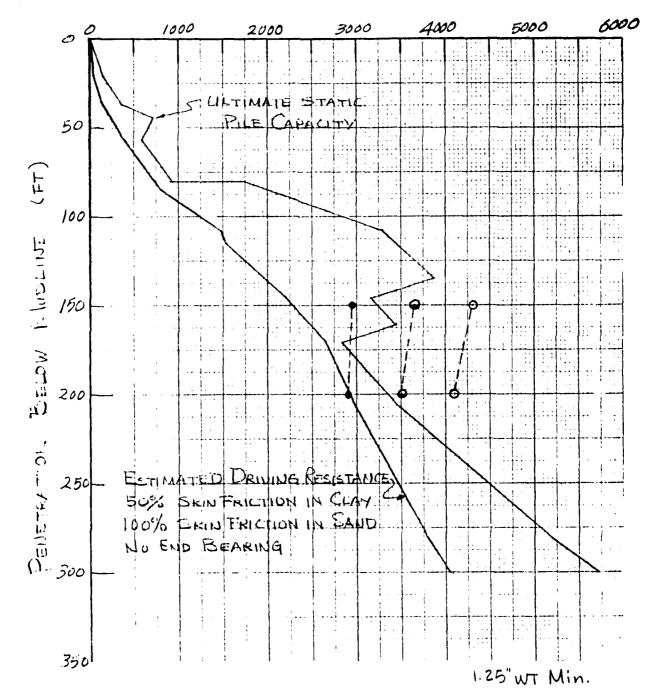


Rate of Penetration, N, Blows per Foot 200-Ft Penetration

Sheet 3.27 of 66__

By C. Character II S. MAUX _ _ subject Foundation Analysis _ _ _ Date & -11-76 Job No. 27-771-97 _ calculation Pile Driving Residence Curres

ULTIMATE STATIC PILE CAPACITY (KIPS) ESTIMATED DRIVING RESISTANCE (KIPS)



Vulcan 560 Hammer

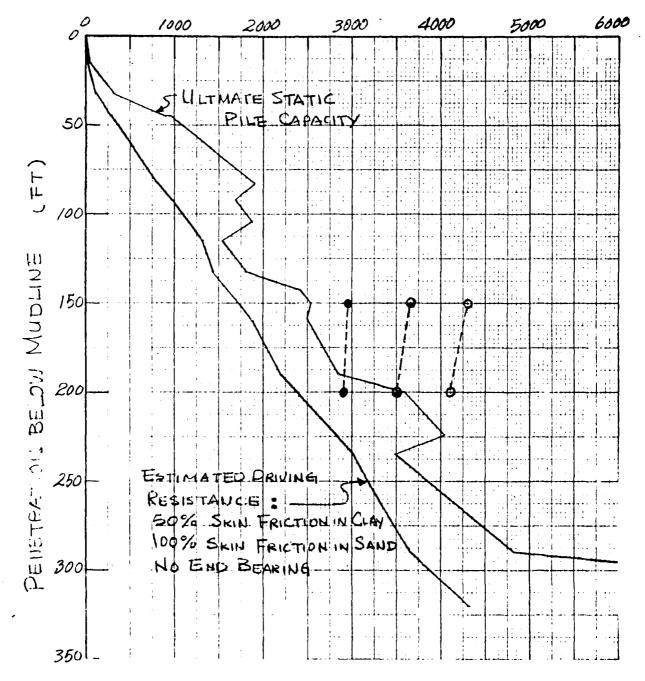
(Boring #1)

42-IN. DIAMETER PIPE PILES

• • • •

By C. Chern Client U. S. NAVY _ subject Foundation Applysis_ Date 6-4-76 Job No. 27-771-91 _ calculation Pile Driving Resistance Curves ULTIMATE STATIC PILE CAPACITY (KIPS)

ESTIMATED DRIVING RESISTANCE (KIPS)



1.25" WT Min.

Vulcan 560 Hammer

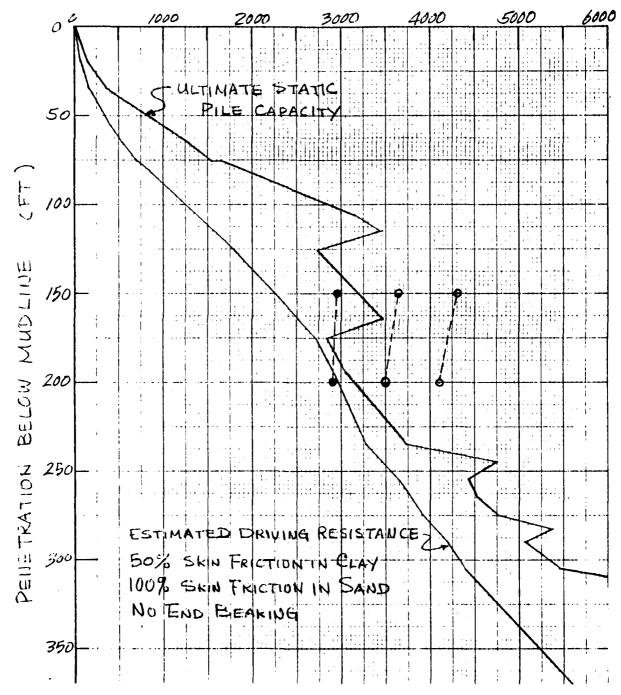
(Boring #2)

42-IN DIAMETER PIPE PILES

Sheet 3.29 of 66

By C. Cherre Client U.S. NAVY _ subject Foundation Analysis ____ Date 6-4-76 Job No. 27-77L-01 _ calculation Pile Driving Resistance Curves

LILTIMATE STATIC PILE CAPACITY (KIPS)
ESTIMATED DRIVING RESISTANCE (KIPS)



1.25" WT Min.

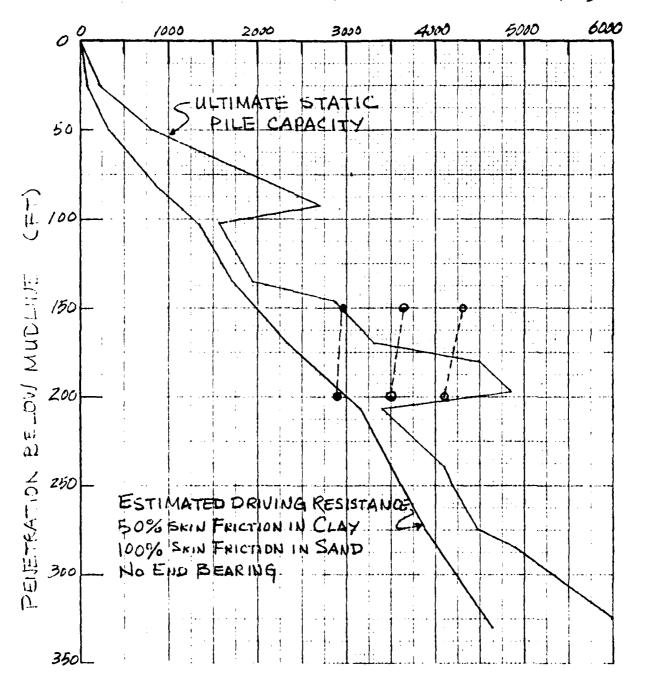
soring 153A) Vulcan 560 Hammer

(Boring #3A)
42-IN. DIATE OR PIPE PILES

Sheet : 30 of 66_

By Signature Client 45. A Subject Tand determine And Subject Tand determine And Subject Tand determine Asia transport to the subject Tand determine Asia tra

ULTIMATE STATIC PILE CAPACITY (KIPS)
ESTIMATED DRIVING RESISTANCE (KIPS)



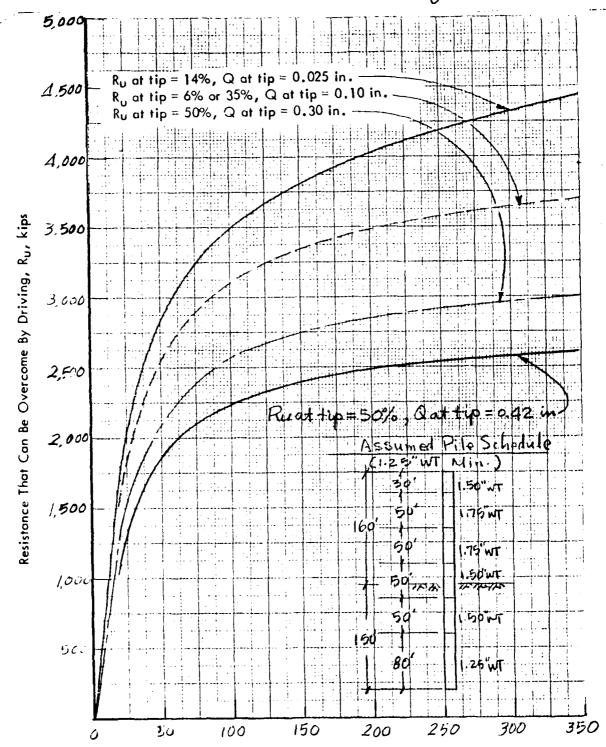
1.25" WT Min.

Videan 560 Hammer

(Boring #4)

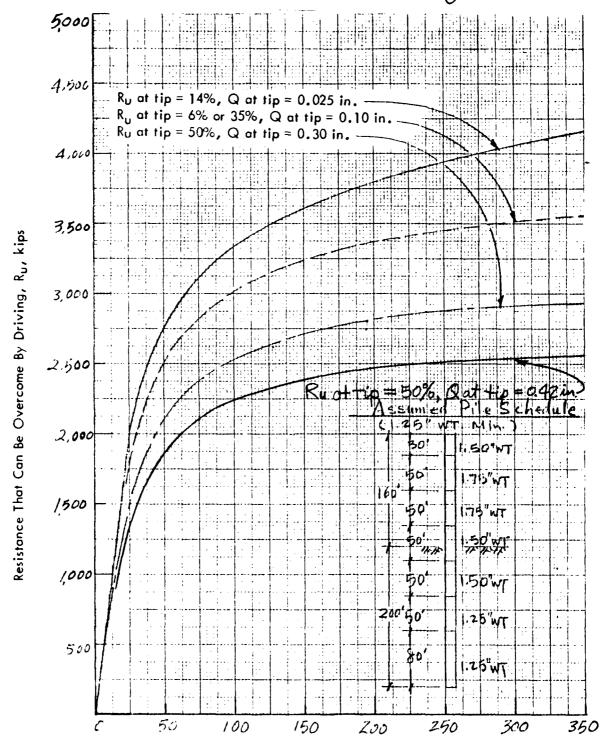
42. IN DIAMETER PIPE PILES

By C. Chern client U.S. NAVY _ _ subject Foundation but wis ____
Date 7-13-26 Job No. 27-771-97 _ calculation Bile Driving Resistance Curves



Rate of Penetration, N, Blows per Foot 150 -Ft Penetration

By C. Cherns client U.S. NAVY _ subject Foundation Analysis __

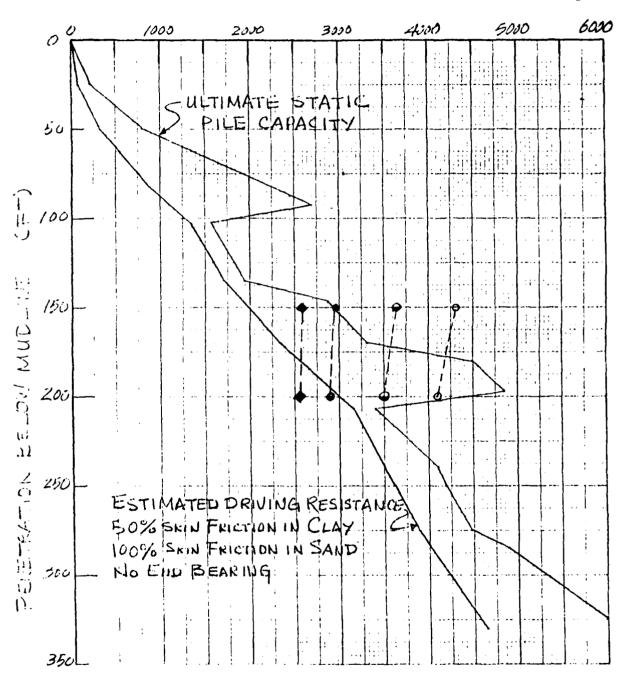


Rate of Penetration, N, Blows per Foot 200-Ft Penetration

Sheet 3.36of 66_

By C. Chern Client U.S. NAUY __ subject Foundation Analysis ___ Date 7-13-76 Job No. 27-771-97 _ calculation Pile Driving Resistance Curves

ULTIMATE STATIC PILE CAPACITY (KIPS)
ESTIMATED DRIVING RESISTANCE (KIPS)



1.25" WT Min.

Valcan 560 Hammer

(Boring #14-)

42 III DIAMETER PIPE PILES

By C. Chern client U.S. NAUY __ subject Foundation Analysis ____ Date 6-10-76 Job No. 27-771-97 _ calculation Bile Driving Resistance Courses _

_1	1					
160,-0"	30-0"			1.75"M	P-6	
	50,00			2.00 "WT	P-5	
11	50-0"		<u> </u>	2.00"WT	p-4	
	50-0"	20,00, 30,02	71577	1.75"WT	p-3	TKT AT AT AT
150,-0"	50-0"			1.75°W	p-2	
	80.00			1.50'WT	1-d	
						-

MLW = 105'-0"
150 FT Penetration

Vulcan 560 Hammer
Wt. of Ram = 60,000 lbs
Rated Energy = 300,000 ft-lbs
Hammer Efficiency = 0.75
Wt. of Pile Cap = 42,000 lbs

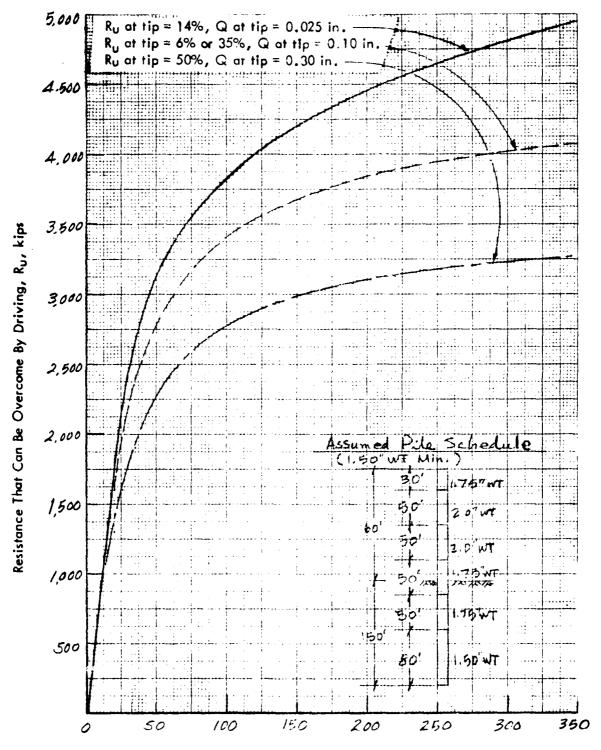
Spring Constant = 6.2 x 10⁶ lbs/in.

Damping Factor, side & tip, J = 0.15

Quake Factor, side, Q = 0.10

Quake Factor, tip, - See Above

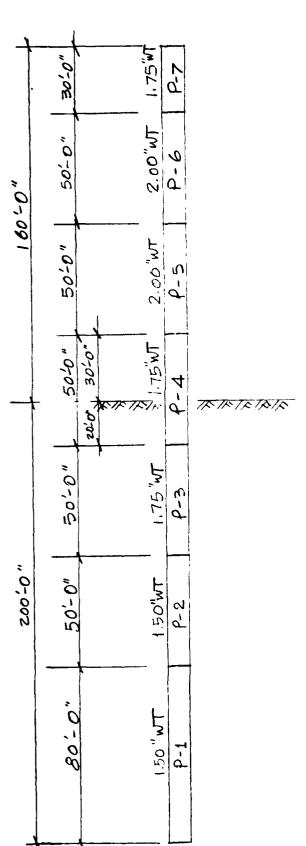
By C. Chern U.S. NAUY __ subject Form dution At a pere Curves_ Date 6=11-76 Job No. 27-771-97 _ calculation 26 Driving Kerickerice Curves_



Rate of Penetration, N, Blows per Foot 150-Ft Penetration

42-IN DIAMETER PIPE PILES

By C. Chern Client U.S. NAUY __ subject Foundation Analysis ___ Date 6-10-76 Job No. 27-771-97 _ calculation Pile Driving Kesistance Curves

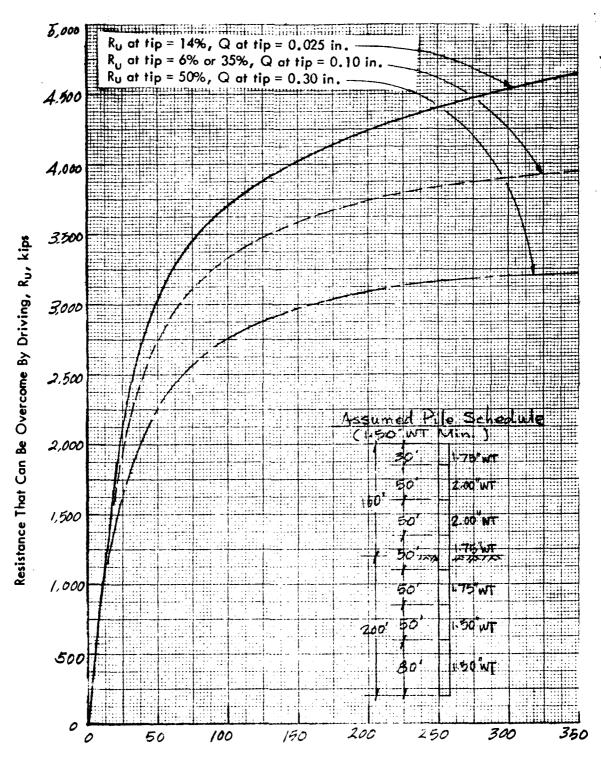


MLW = 105'-0"

200 FT Penetration

Vulcan 560 Hammer Wt. of Ram = 60,000 lbs Rated Energy = 300,000 ft-lbs Hammer Efficiency = 0.75 Wt. of Pile Cop = 42,000 lbs

Spring Constant = 6.2×10^6 lbs/in. Damping Factor, side & tip, J = 0.15Quake Factor, side, Q = 0.10Quake Factor, tip, - See Above By C. Chern Client U.S. NAUY subject Foundation Analysis ____ Date 6=11-ZE Job No. 27-77L-97 calculation Pile Driving Resistance Curves



Rate of Penetration, N, Blows per Foot 200-Ft Penetration

42-IN. DIAMETER PIPE PILES

OFFSHORE, INC. **CREST**

Sheet 3.42 of _66_

By C. Chern client U.S. NAVY __ subject Foundation Analysis____ Date 6-25-76 Job No. 27-771-97 _ calculation Pile Driving Resistance Curves

MLW = 105'-0"

250 FT Penetration

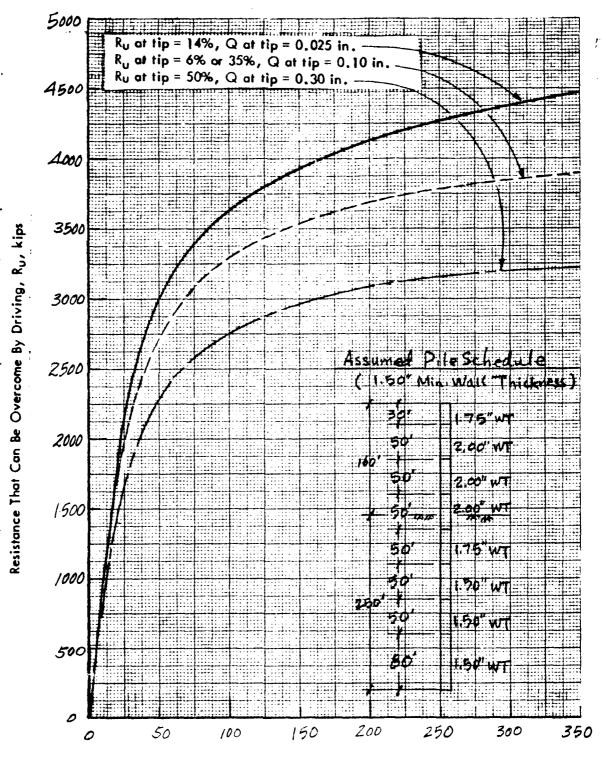
Vulcan 560 Hammer Wt. of Ram = 60,000 lbs Rated Energy = 300,000 ft-lbs Hammer Efficiency = 0.75 Wt. of Pile Cap = 42,000 lbs

Spring Constant = 6.2×10^6 lbs/in. Damping Factor, side & tip, J = 0.15 Quake Factor, side, Q = 0.10Quake Factor, tip, - See Above

1.50"WT

By C. Cher U.S. NAUX _ subject Foundation Analysis.

Date 6-25-76 Job No. 27-771-97 _ colculation Pile Driving Resistance Curves

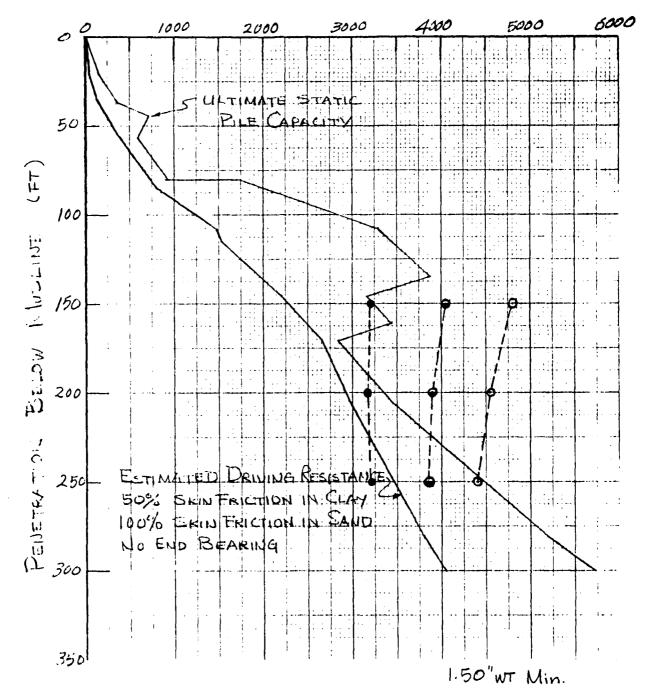


Rate of Penetration, N, Blows per Foot 250-Ft Penetration

Sheet 3.44 of 66

By C. China Client U.S. MAUX ___ subject Foundation Analysis ____ Doto G. L. L. Coloulotion Pile District Colours Colours Colours Colours China Estimate Static Pile Capacity (KIPS)

Estimated Driving Resistance (KIPS)



Vulcan 560 Hammer

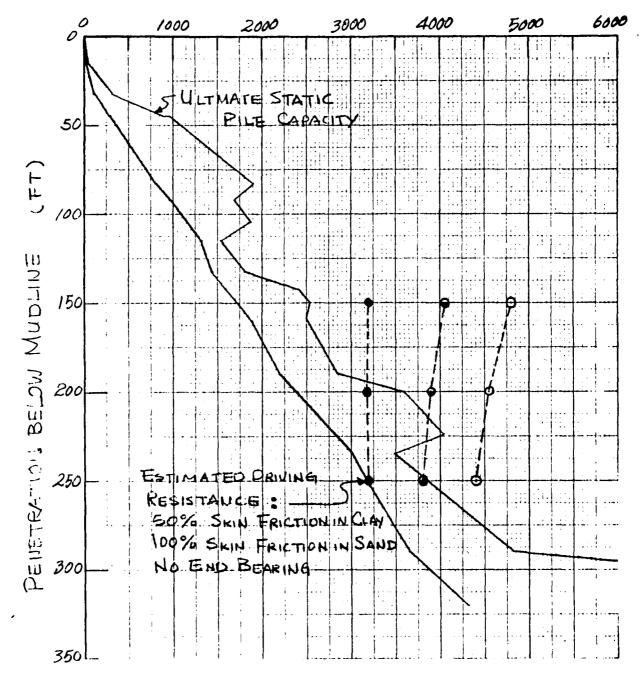
(Boring #1)

42-IN DIAMETER PIPE PILES

By C. Chern client U.S. NAVY __ subject Foundation Analysis _____
Date 6=4-76 Job No. 27-771-91 _ calculation Pile Driving Resistance Curves

ULTIMATE STATIC PILE CAPACITY (KIPS)

ESTIMATED DRIVING RESISTANCE (KIPS)



1.50" WT Min.

Vulcan 560 Hammer

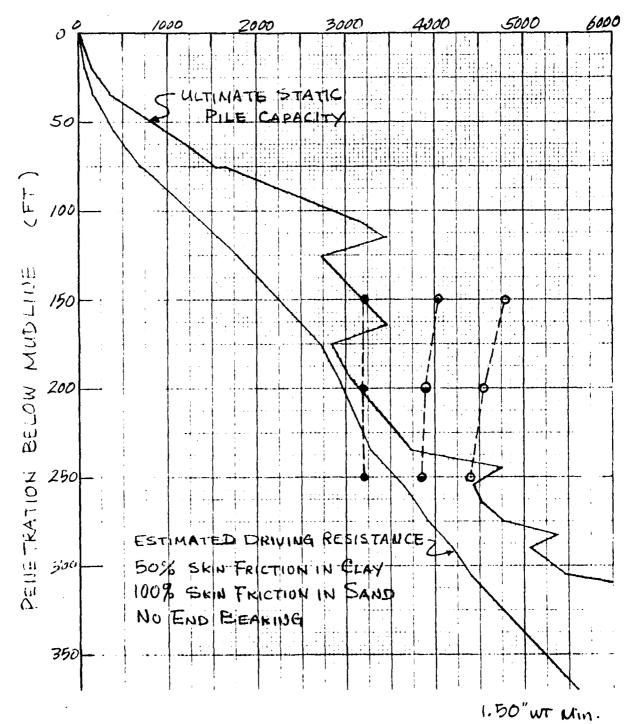
(Boring #2)

42-IN DIAMETER PIPE PILES

Sheet 2:4601 66_

By C. Cherr Client U.S. NAVX _ subject Foundation Acalysis _ ___ Date 6-4-76 Job No. 27-771-91 _ calculation Pile Driving Resistance Curves

LILTIMATE STATIC PILE CAPACITY (KIPS)
ESTIMATED DRIVING RESISTANCE (KIPS)



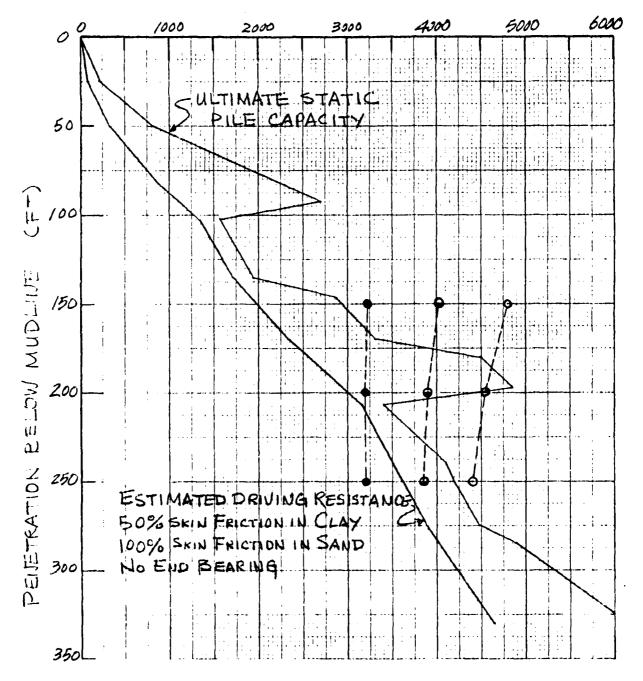
(Boring #3A)
42-IN DIAMELER PIPE PILES

Vulcan 560 Hammer

Sheet 3:47ot 66

By C. Cherry Client U. S. NAVY _ subject Foundation Analysis _ ____ Date 6-4-76 Job No. 27-771-91 _ calculation Pile Driving Resistance Carves

ULTIMATE STATIC PILE CAPACITY (KIPS)
ESTIMATED DRIVING RESISTANCE (KIPS)



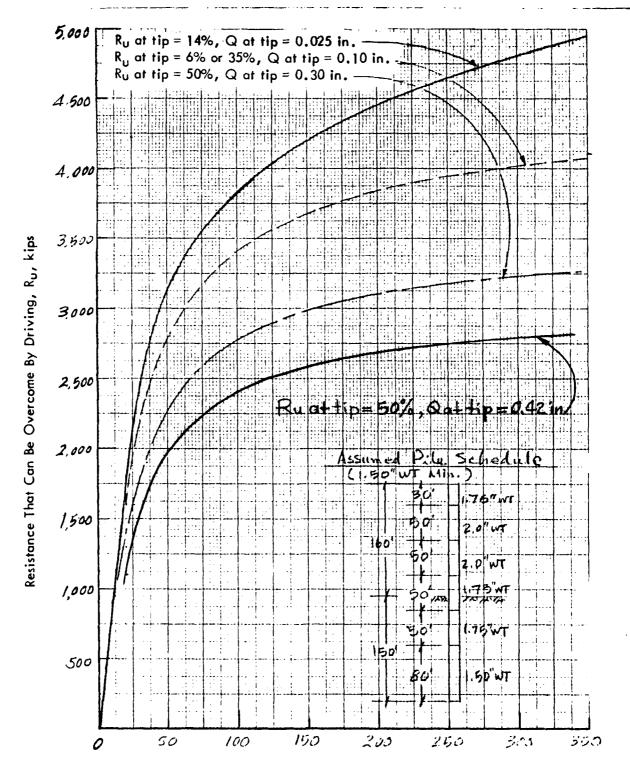
1.50" WT Min.

Vulcan 560 Hammer

(Boring #4)

42. IN DIAMETER PIPE PILES

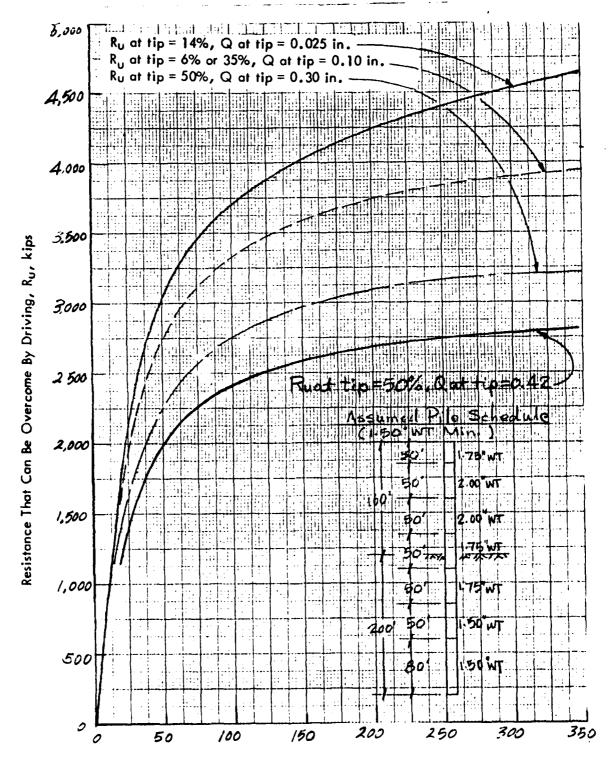
By C. Chern client U.S. NAUY __ subject Foundation Analysis ____ Date 7-13-26 Job No. 27-771-97 _ calculation Pile Driving Resistance Curves



Rate of Penetration, N, Blows per Foot 150-Ft Penetration

42-10 DIAMI TER PIPE PILES

By C. Chern_ client Ll. S. NAUY_ subject Foundation Analysis_____
Date 7=13-26 Job No. 27-72 L-27_ calculation Pile Driving Resistance Chaves



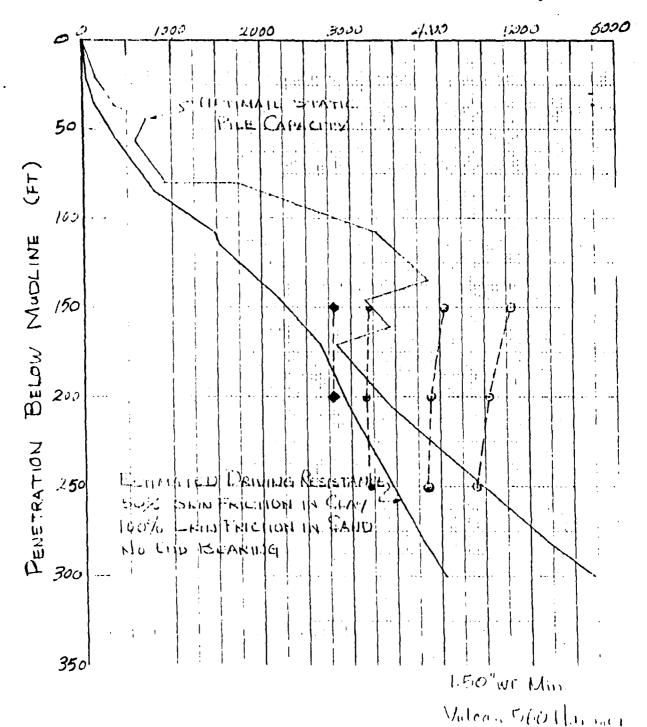
Rate of Penetration, N, Blows per Foot 200-Ft Penetration

42-IN. DIAMETER PIPE PILES

Sheet _ 50 of 66__

By C. Chern_ client U.S. NAUX _ subject Foundation Anolysis ____ Date 7-13-26 Job No. 27-771-97 _ calculation Pile Driving Resistance Curves

HUTIMATE STATIC PILE CAPACITY (KIPS)
LITIMATED DRIVING RESISTANCE (KIPS)



(Biring #1)

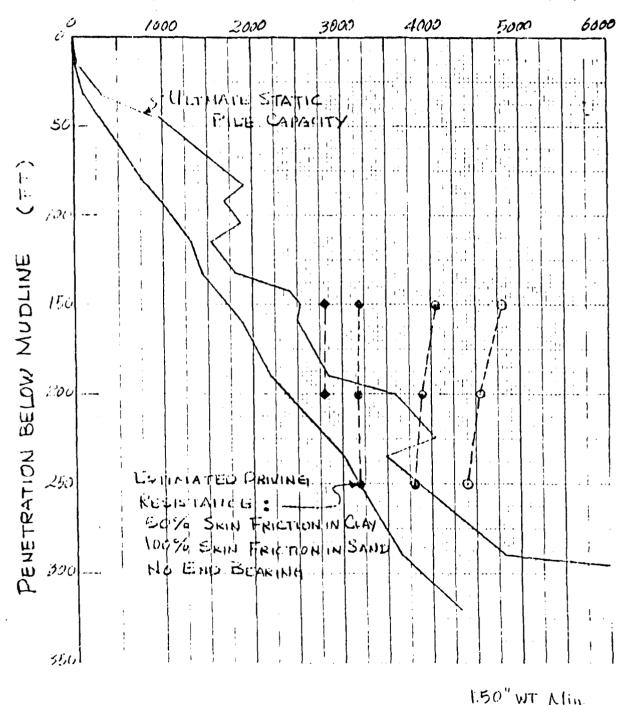
42 m DUMETER PIPE PILES

OFFSHORE, INC. **CREST**

Sheet 3.51 of 66_

By C. Cheri Cilent Ll. S. NAVY _ _ subject Foundation Analysis ____
Date 7-13-76 Job No. 27-771-91 _ calculation Pile Driving Resistance Cares

MUTIMATE STATIC PILE CAPACITY (KIPS) ESTIMATED DRIVING RESISTANCE (KIPS)



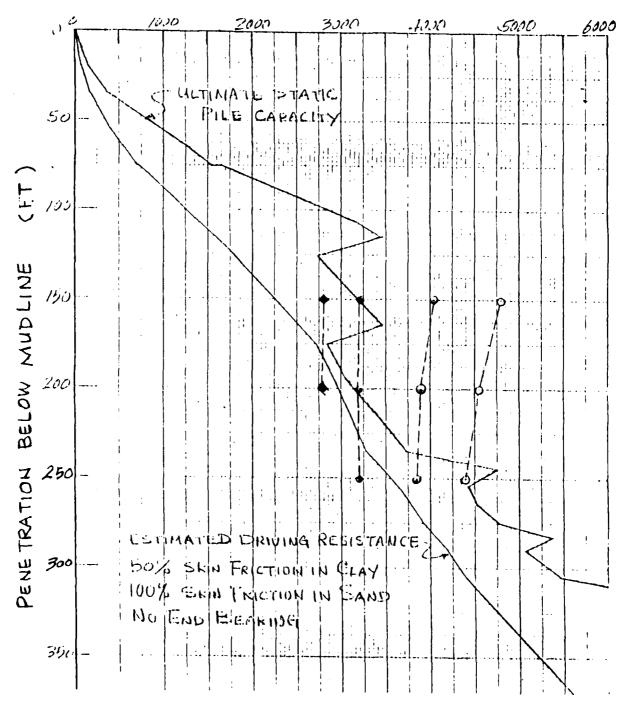
(150 ring 12)

Vilous 500 Hammer

12 IN DI NETER PIPE PILES

By C. Cherry Cilent U.S. NAUX_ subject Foundation Augustican Date Z-13-26 Job No. 27-771-97 calculation Bile Driving Resistance Curves

LICHMATE STATIC PILE CAPACITY (KIPS)
ESTIMATED DRIVING RESISTANCE (KIPS)



1.50" we show

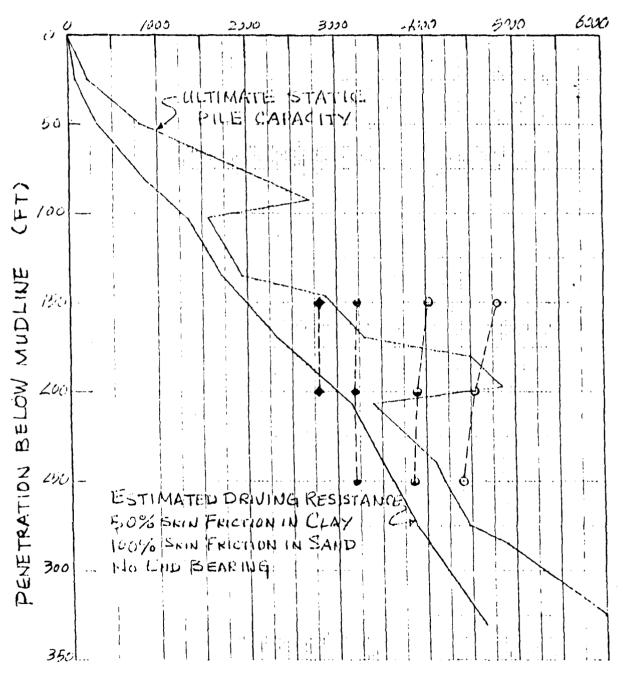
Valence Con Hammer

(Boing #3A)
42-11) DIATH OR PIPE PILES

Sheet = 201 66_

By C. Chern Client U.S. NAVY _ subject Foundation And Price Convest Date 7=13-76 Job No. 27-771-97 _ colculation Pile Driving Action use Convest

LILIMATE STATIC PILE CAPACITY (KIPS)
ESTIMATED DRIVING RESISTANCE (KIPS)



1.50" w. Min.

Valence Till Hamor

(Boring 11-1-)

42 m. DIAME OR PIPE PILES

3.6 PILE SCHEDULE NO. 4 -- 2.00 IN.
UNIFORM WALL THICKNESS

3.6 PILE SCHEDULE NO. 4 -- 2.00 IN.
UNIFORM WALL THICKNESS

Sheet 225 66_

By C. Cherry client U.S. NAVY __ subject Finandation Analysis _____ Date 6-25-76 Job No. Z7-771-97 _ calculation Pile Driving Resistance Curves

MLW = 105'-0"

250 FT Penetration

Vulcan 560 Hammer Wt. of Ram = 60,000 lbs Rated Energy = 300,000 ft-lbs Hammer Efficiency = 0.75 Wt. of Pile Cap = 42,000 lbs

Spring Constant = 6.2 x 10⁶ lbs/in.

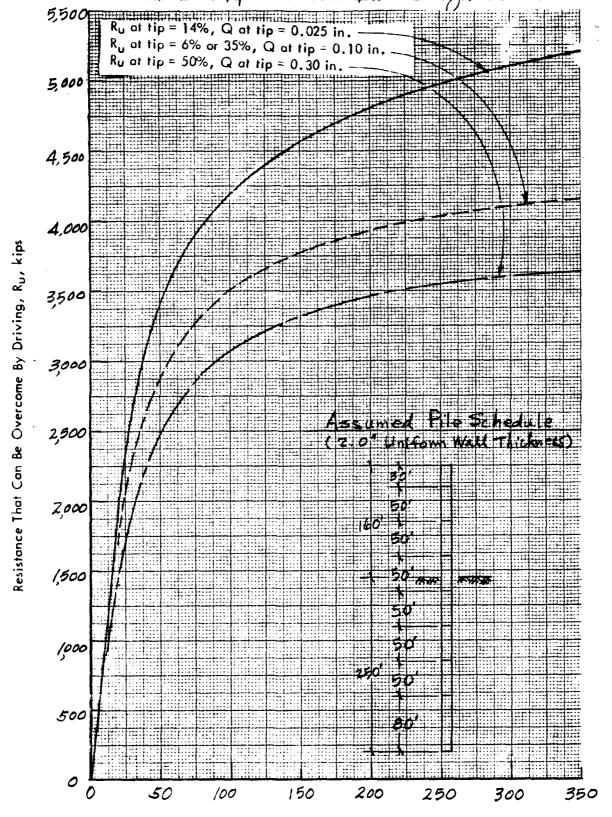
Damping Factor, side & tip, J = 0.15

Quake Factor, side, Q = 0.10

Quake Factor, tip, - See Above

•	J			
	30,0"	2.0°ET	8-8	
160'-0"	,0,00 "0,05	2.0 "UT 2.0 "UT	P-7	
	20,00	2.0"WT	P-6	
	50-0	70.00 30.00	P-5	<i>\(\overline{\pi} \)</i>
250'-0"	20,00	20.00 F3.00	p-4	
	"0-25 "0-25	2.0°	P-3	
	"0-05	2.0°X	P.2	
	80'-0"	TX" 0.5	1-0	

By C. Cherr- client U.S. NAW __ subject Four dation Aralysis____
Date 6-23-76 Job No 27-771-911 _ calculation Pile Driving Residence Curves

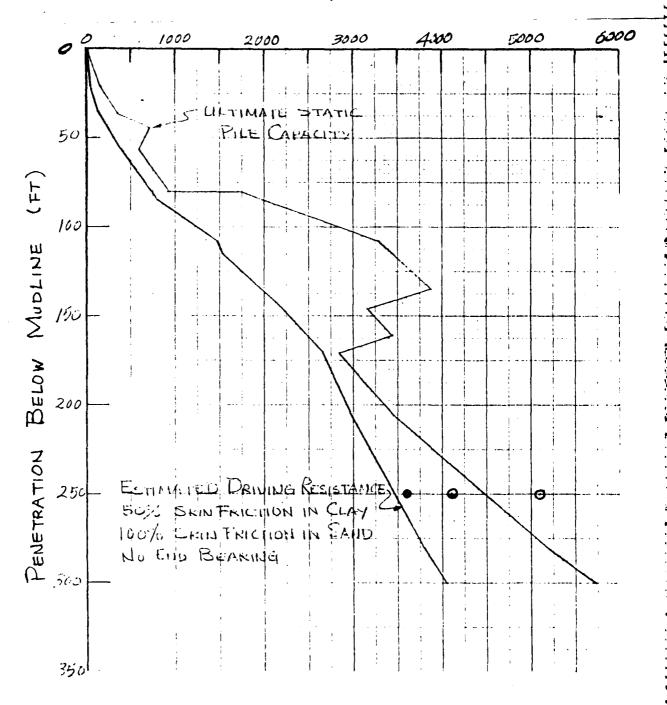


Rate of Penetration, N, Blows per Foot 250-Ft Penetration

Sheet 3 2 Zot 66_

By C. Chern Client U.S. NAUX __ subject Foundation Airelysis ____ Date 6-28-76 Job No. 27-771-97 _ calculation Pile Driving Resistance Curves_

ULTIMATE STATIC PILE CAPACITY (KIPS)
ESTIMATED DRIVING RESISTANCE (KIPS)



(Boring #1)

Z-in Uniform Wall

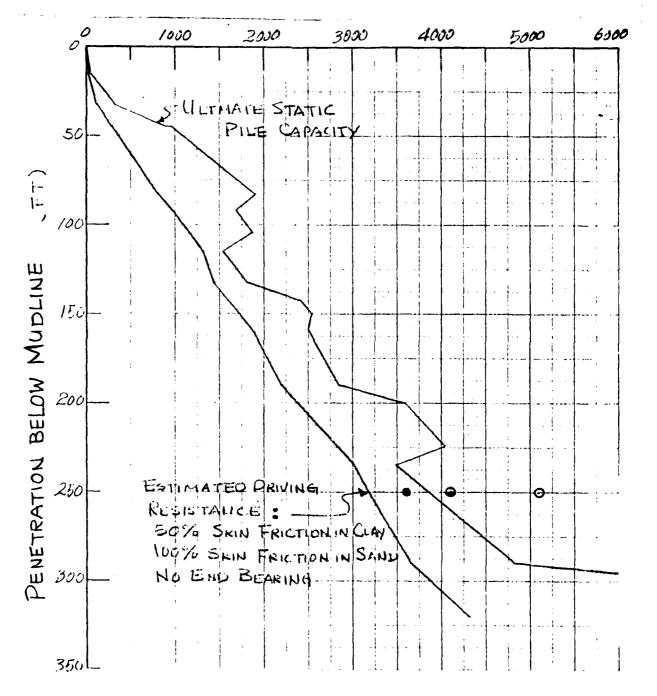
42-111 DIAMETER PIPE PILES

CREST OFFSHORE, INC.

Sheet = FBot 66_

By C. Cherry Client U.S. NAUX __ subject Foundation Aralysis_____
Date 6-28-76 Job No. 27-271-97 _ calculation Pile Driving Resistance Curves

ULTIMATE STATIC PILE CAPACITY (KIPS) ESTIMATED DRIVING RESISTANCE (KIPS)



2-in Uniform Wall

(Bering #2)

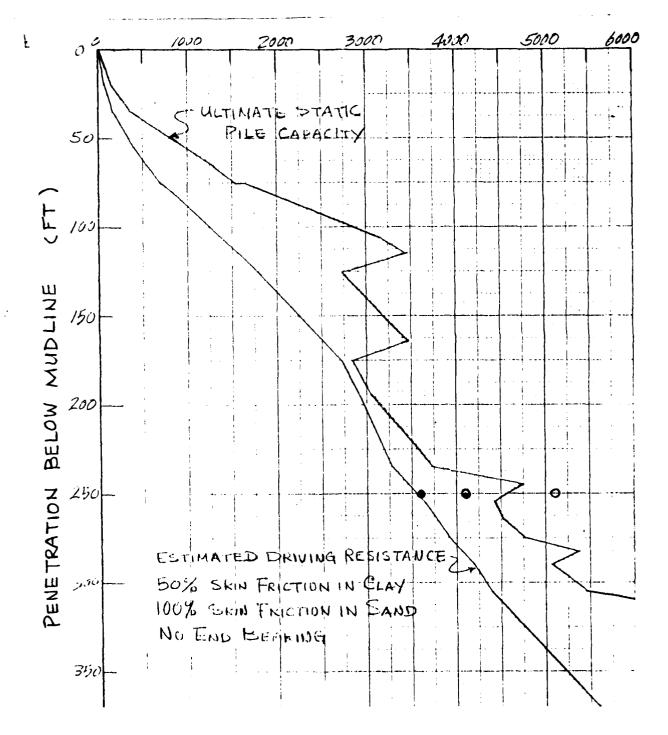
42 IN DIAMETER PIPE PILES

CREST OFFSHORE, INC.

Sheet = 2901 66_

By C. Cheri _ client U.S. NAVY _ _ subject Foundation Arzaris _ _ _ Date 6-28-76 Job No. 27-771-97 _ calculation Pile Driving Resistance Curves

ULTIMATE STATIC PILE CAPACITY (KIPS) ESTIMATED DRIVING RESISTANCE (KIPS)



2-in. Uniform Wall

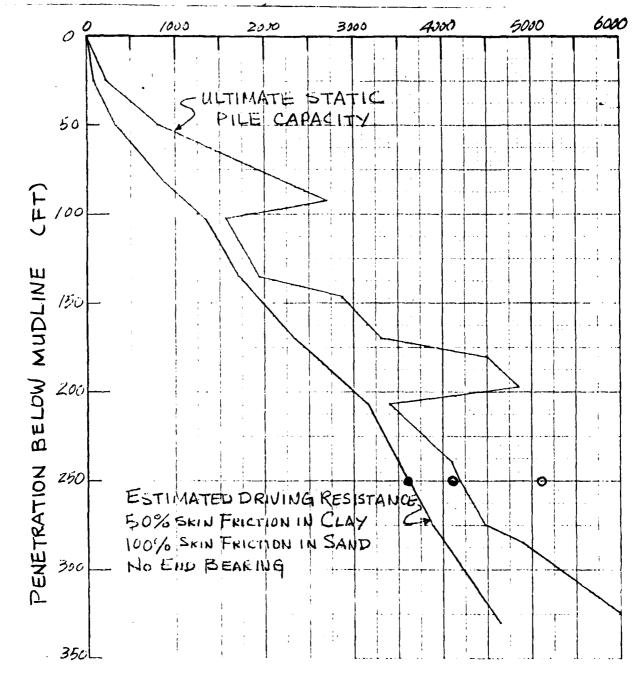
(Boring "3A)
42-IN DIAMELOR PIPE PILES

Sheet = 160of _66_

By C. Chern Client U.S. NAVY __ subject Foundation Analysis ____
Date 6-28-76 Job No. 27-771-97 _ colculation Pile Driving Resistance Curves

ULTIMATE STATIC PILE CAPACITY (KIPS)

ESTIMATED DRIVING RESISTANCE (KIPS)



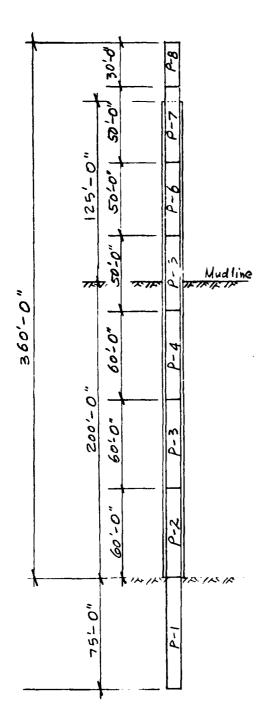
Z-in. Uniform Wall

(Boring #4)

42-111 DIAMETER PIPE PILES

3.7 33 IN. DIAMETER INSERTED PILING

By C. Chern client U.S. NAVY ___ subject Foundation Analysis ____ Date 8-25-76 Job No. 27-771-97 calculation Pile Driving Resistance Curves



MLW = 105'-0"

42' Piling at 200 FT Penetration

33' Piling at 75 FT Penetration

Vulcan 040 Hammer
Wt. of Ram = 40,000 lbs
Rated Energy = 120,000 ft-lbs
Hammer Efficiency = 0.75
Wt. of Pile Cap = 27,800 lbs

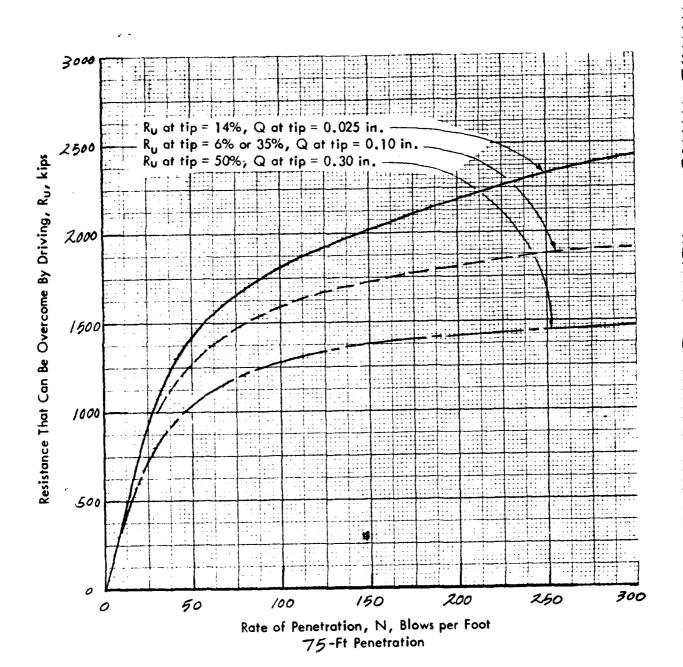
Spring Constant = 2.78 x 10⁶ lbs/in.

Damping Factor, side & tip, J = 0.15

Quake Factor, side, Q = 0.10 in.

Quake Factor, tip, - See Above

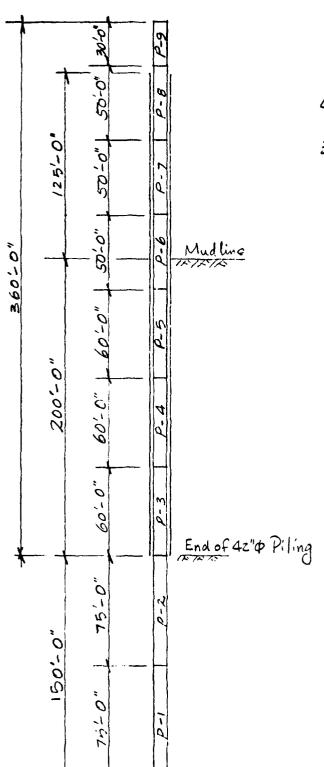
By C. Chern client U.S. NAUY __ subject Foundation Analysis _____
Date B-26-76 Job No. 27-771-97_ calculation Pile Driving Resistance Carves



33-IN. DIAMETER INSERTED PILING

By C. Cherm Client U.S. NAVY _ subject Foundation Anniques

Date 8-25-76 Job No. 27-771-97 _ calculation Pile Driving Resistance Conves



MLW = 105'-0"

42" & Piling at 200 FT Penetration

33" & Piling at 150 FT Penetration

Vulcan 040 Hammer
Wt. of Ram = 40,000 lbs
Rated Energy = 120,000 ft-lbs
Hammer Efficiency = 0.75
Wt. of Pile Cap = 27,800 lbs

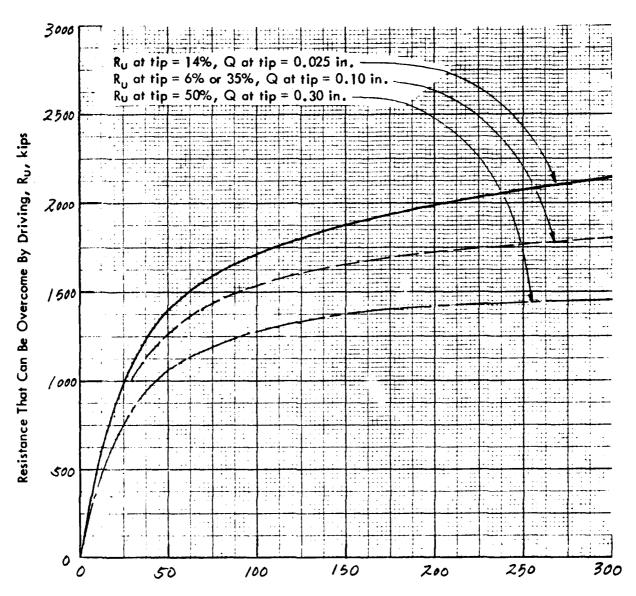
Spring Constant = 2.78 x 10⁶ lbs/in.

Damping Factor, side & tip, J = 0.15

Quake Factor, side, Q = 0.10 in.

Quake Factor, tip, - See Above

By C. Cheriz client U.S. NAUY __ subject Foundation Analysis ____ Date 8-26-76 Job No. 27-171-97 _ calculation Pile Driving Resistance Cusue

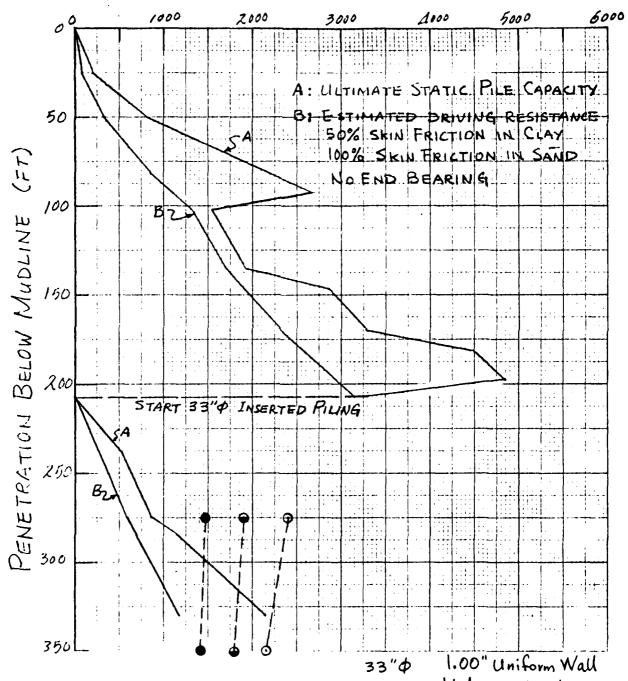


Rate of Penetration, N, Blows per Foot
150-Ft Penetration

33-IN. DIAMETER INSERTED PILING

By C. Cher: client_U.S. NAVY_ subject Foundation Analysis_ Date 8-26-76 Job No. 27-771-97 calculation Pile Driving Resistance ULTIMATE STATIC PILE CAPACITY (KIPS)

ESTIMATED DRIVING RESISTANCE (KIPS)



Vulcan 040 Hammer 42-IN. DIAMETER PIPE PILES (200 FT PENETRATION)

33-IN DIAMETER INSERTED PILES (Boring #4)

APPENDIX

STRESS-WAVE ANALYSIS

Pile Driving Resistance Curves

Pile Diameter - 42 in.

Minimum Wall Thickness -1.25 in.

Penetration - 150 ft.

- 200 ft.

Hammer - Vulcan 560

Quake Factor, Tip - .42 in.

BORING SITES 3A SPRING CONSTANT 00000029 42-1N. DIAMETER PILES MLWS105FT 3-PILE STRUCTURES 150FT PENETRATION ** VULCAN 560 HAMMER OTIPE,420,MINIMUM WALL THICKNESS\$1,25 IN. RU = 5: CAPEA COEF OF 900 WAVE EQUATION ANALYSIS FOR 42-IN, DIAMETER PIPE PILES MC CLELLAND SQIL REPORT DATA FOR ACMR 3-PILE STRUCTURES 13 JULY 1976 VULCAN SOO HAMMER 490.0 24000000 MODOL US (BPF) ---00 3000000,00 00.0 NEW MATERIAL DATA OPTION

NEW DILE SECTION DATA CPTION

SPECIFIED BLOW COUNT OPTION

CUTPUT OPTION TOR STRESS

BPF FOR STRESS CUTPUT OPTION

CLITATE RESISTANCE INCREMENT (TONS)

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WAS SLOWS FO -I -I -I 42000,00 HEIGHT (LB) HANNER DESCRIPTION
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HANNER ENERGY (FTGLBS)
HANNER EXPLOSIVE FORCE (LBS)
NUMBER OF MANNER SEGMENTS NUMBER OF NATERIAL TYPES & -- PROGRAM CONTROL DATA 45,000 (100) TABLE 4 .. PILE SECTION DATA PILE TYPE New Hammer Data Option 1000 .. MATERIAL DATA SLACK (IN) TABLE 2 .. HAMMER DATA MATERIAL アンボルいじ アンドラインド アンドラインド TABLE 3 TABLE

TOTAL NUMBER OF PILE SECTIONS

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7052 5737 4541 3439 2420 641 143 MLME105FT S-PILE SEG MAX TEBS/SO, IN. MAX C STRESS LBS/SG.IN, NO. 8326 8354 8354 8354 8375 8375 8375 OTIPE,420,41414UM MALL THICKNESSA1,25 -- RESISTANCE-BLUM CURVE DATA REGISTANCE DYNAMIC PT TOTAL-TONS FUNCE-TONS RESISTANCE PENCENTAGE 750 BLOBS/FT. • TABLE

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*AVE EGUATION ANALYSIS FOR 42-IN. DIAMETER PIPE PILES HC CLELLAND SGIL REPORT DATA FOR ACHR 3-PILE STRUCTURES =- BORING SITES 3A 13 JULY 1976 SPRING CONSTANT (LB / IN) RU # 50 42-10 DIAHETER PILES HEMBIOSFT 3-PILE STRUCTURES 200FT PENETRATION -- VULCAN 540 HAMMER 601P*420,MINIMUM MALL THICKNESS#1,25 IN. RU # 5 30°0 30°0 30°0 AREA COEF OF (SO IN) RESTITUTION 96 VULCAN SOO HAMMER MODULUS 496,0 29000000 ULTIMATE RESISTANCE INCREMENT (TONS)
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SPECIFIED SEGMENT LENGTH (FT) 000 (PCF) 42000,00 NEW HAMMER DATA OPTION
NEW HATERIAL DATA OPTION
NEW PILE SECTION DATA CPTION
NEW SOIL DATA OPTION
SPECIFIED BLOW COUNT OPTION
CUTPUT CPTION FOR STRESS WEIGHT (LB) HPF FOR STRESS QUIPUT CPITUN (1.88) ** PROGRAM CONTROL DATA HAMMER DESCRIPTION
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HAMMER ENERGY (FTGLB9)
MAMMER EXPLOSIVE FORCE (LB 42,000 NUMBER OF MATERIAL TYPES (100) TABLE A .- PILE SECTION DATA 1000,00 .. MATERIAL DATA DESCRIPTION SLACK (IN) SE HAMMER DATA MATERIAL BEGHEN4 NUMBER TABLE 1 TABLE 3 PROB

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23

TOTAL NUMBER OF PILE SECTIONS

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2007 PENETRATION -- VULCAN 560 MAMMER

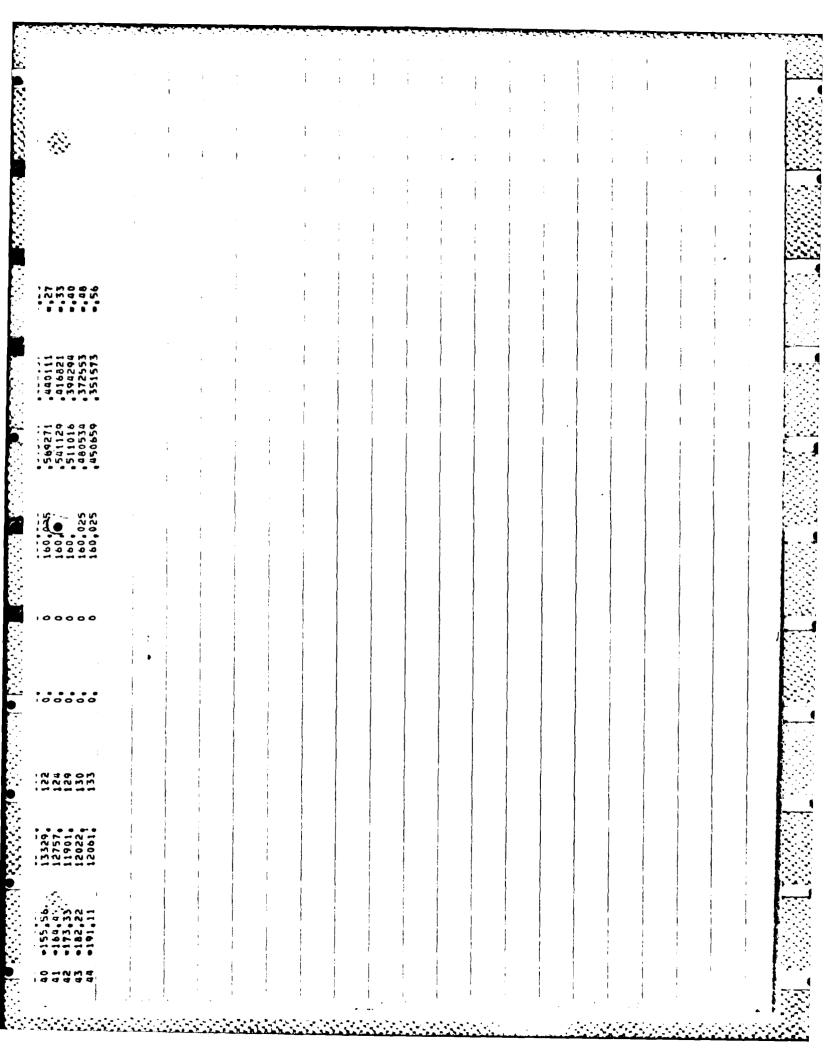
RU = 50 OTIPE.420, MINIMUM WALL THICKNESS=1.25 IN.

TABLE & ... MAXIMUM STRESS DATA

50.00 TIP RESISTANCE PENCENTAGE .

.0307 INCHES PERMANENT SET OF PILE & NUMBER OF SLOWS

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•	TRATION 9	TABLE 10 44 SPECIFIED BLUW DATA	TIP RESISTANCE PERCENTAGE	BLU#S PER FOCT	156,37	227.65 280.63						

Pile Driving Resistance Curves

Pile Diameter - 42 in.

Minimum Wall Thickness -1.50 in.

Penetration - 150 ft.

- 200 ft.

Hammer - Vulcan 560

Quake Factor, Tip - .42 in.

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BORING SITES 3A SPRING CONSTANT 42-1N, DIAMETER PILES MLWA105FT 3-PILE STRUCTURES 150FT PENETRATION -- VULCAN 560 HAMMER GIPE, 420, MINIMUM MALL THICKNESS#1,50 IN, RU # 51 MAVE EQUATION ANALYSIS FUR 42-IN, DIAMETER PIPE PILES MC CLELLAND SQIL REPORT DATA FOR ACMR 3-PILE STRUCTURES --275 500 300 000 AREA COEF OF (80 IN) RESTITUTION 960 VULCAN 1560 HARRINA 490,0 29000000 CUTPUT CPTION FOW STRESS
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6 -141,11 1000,00 5772,69 190,65 1,00 5168794	8 - 141,111 - 1000,00 - 5772,60 100,65 1,00 5108744	# 14	794
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•PILE STRUCTURES					A DHAX(H) D(H) V(H) IN, FT/SEC	1,433027 1,433027 •.0 1,001405 .965276 •.8	981987 941531 69	2010 0000000000000000000000000000000000	19 1/2070	926030 871704	907966 850531	.090404 .000463 .01. .001773 .0026596 .1. .005179 .001410 .1.	880181 800239 •1. 873478 786538	865887 172280	.04757 775693 ele	.822015 .700190 =1.	.807922 .680639	775399 .641621	717080 6603813 e.	6 1 1 1 2 2 2 2 2 3 3 3 3 3 3 3 3 3 3 3 3	659521 557621	613941 482454	590872 460670 5	.539593 .418695	2 483469 878996 8.59 8.59 8.59 8.59 8.59 8.59 8.59 8.59	
1 42-IN DIAMETER PILES MLWB105FT 3-	O, MINIMUM WALL THICKNESSEI, 50 IN. RU # 50	KINUM STRESS DATA	STANCE PERCENTAGE # 50.00	T SET OF PILE # ,0371 INCHES F SLOAS PER FOOT # 323,88 TERVALS # 156	RESS TIME N MAX T STRESS TIME N AREA IN.	5 0 121 1	9 0 0 221	22 0 0 221		2 0 0 251	0 0 251.		5 0 0 251	251		221	1 0 0 221	5 0 0 221	9 0 0 221	22.10	001			3 0 100		THE RESERVE AND ADDRESS OF THE PARTY OF THE
PROB 150FT PENE	.011Pa,420	TABLE 8 ** MAXI	TIP RESIST	PERMANENT NUMBER OF TOTAL INTE	SEG ELEV MAX C STR	0.00 412	150,00	130,00	121	96,67	6,33	71.67	46.67	36,33	3.33	5,00	3 +11,67	28,33	7 - 45,00	9 -61,67	-70.00	87.78	-105,56	114,44	#132,22 #141,11	

BOOK OFFICE SHOWS BOOKERS

ABLE 9 == RESISTANCE BLUM CURVE DATA TIP RESISTANCE PENCENTAGE = 50,00 BLOMS/FT, RESISTANCE DYNAMIC PT MAX C STRESS SEG MAX T STRESS SEG 1,75
The SISTANCE PERCENTAGE E 50,000 The SISTANCE CYNAMIC PT MAX C STRESS SEG MAX T STRESS SEG TOTAL TOTAL TONS LUS/SQ.IN. NO. LU
TO RESISTANCE DYNAMIC PT MAX C STRESS SEG MAX T STRESS SEG TO STRESS SEG MAX T STRESS SEG TO STRESS SEG MAX T STRESS SEG TO STRESS SEG MAX T STRESS SEG TO STRESS SEG MAX T STRESS SEG TO STRESS SEG MAS TO STRESS
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33 300, 458,65 16786, 30 7916, 30 46 350, 517,22 16797, 30 6804, 30 54 400, 571,70 16807, 30 5787, 30 55 450, 622,80 16816, 30 4846, 30
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14,75/ 1350, 1066,09 16931, 30 0.
3,88 1400, 1056,75 16931, 30 0,

150FT PENETRATION ** VULCAN 560 HANKER GTIPE 420, MINIMUM WALL THICKNESSEI, 50 IN. TIP RESISTANCE PERCENTAGE B TABLE 10 .. SPECIFIED BLUM DATA BLUMS PER FOOT 157,23 214,75 241,68 323,88 MAVE EQUATION ANALYSIS FOR 42-IN, DIAMETER PIPE PILES MC LLELLAND SOIL KEPORT DATA FOR ACMR 3-PILE STRUCTURES -- BORING SITES 3A 13 JULY 1976

STANSON PERSONNEL E

RU # 50 42-IN, DIAMETER PILES MERHIOSFT 3-PILE STRUCTURES 200FT PENETRATION +* VULCAN 5-0 HAMMER COLLAGO, MINIMUM MALL THICKNESSET, 50 IN, RU # 50 PROB 2

TABLE 1 -- PROGRAM CONTROL DATA

							275,	50.0	300	00.00
		:			-	-		ICNS)	CURVE (8PF)	
	CHILDN	NULL NULL NULL NULL NULL NULL NULL NULL	DATA CPTION	110%	BLOW COUNT OPTION	DR STRESS	UTPUT CPTION	INCE INCREMENT (TONS)	₹	IT LENGTH (FT)
PILE TYPE	NEW HAMPER DATA CPTION	NEW MATERIAL DATA OPTION	NEW PILE SECTION	NEW SOIL DATA OPTION	SPECIFIED BLOW C	CUTPUT OPTION FOR STRESS	HPF FOR STRESS C	ULTIMATE REGISTANCE INCREMEN	MAX BLOKS FOR RESISTANCE-BLOM	SPECIFIEU SEGMEN
_	_	-	_	_		_	_		-	-,

TABLE 2 .. HANNER DATA

	AREA COEFOF SPRING CONSTANT (SO IN) RESTITUTION (LB / IN)	00.000000000000000000000000000000000000
E E	COEF OF RESTITUTION	90
VULCAN 560 НАММЕЯ 300000,00 •0,00	AREA (SO IN)	11000
(L69) 175	WEIGHT (LB)	1000,000 60000,000
HANNER DESCRIPTION NAMMER EFFICIENCY NAMMER ENERGY (FTGL88) TAMMER EXPLOSIVE FURCE (EBS) NUMBER OF NAMMER SEGMENTS	SLACK	1000,000
	S S S S S S S S S S S S S S S S S S S	-~

TABLE 3 . MATERIAL DATA

NUMBER OF MATERIAL TYPES #

MODULUS	29000000
CADE)	0.064
(100)	42,000
MATERIAL	**

TABLE 4 .. PILE SECTION DATA

TUTAL

(<u>`</u>	STATION NUMBER TOP BOTTOM	30 30 30 30 30 30 30 30 30 30 30 30 30 3	:									
0 E(FT) 160,00	CKNESS LENGTH	2 000 2 000 2 000 1 750 1 750 1 500 800 800		PERCENTAGES 1 E - JSIOE 15 CE - JPONT 15 CE - OSIDE 10		COUNT DATA	COUNTS	9 0	25. 25. 25.	•6		
OF SECTIONS ADDED OF FREE STANDING PILE(FT)	HATERIAL WALL THICKNES		0ATA	NUMBER OF TIP RESISTANCE PER SIDE DAMPENING RESISTANCE POINT DAMPENING RESISTANCE & SOIL QUAKE FOR SIDE	TIP RESISTANCE PERCENTAGE SO.0000	IFIED BLUM	SPECIFIED BLUM	PER TULERANCE				
AD HIT IN	SECTION	~ MN 4 N 4 F	TABLE 5 SOIL	NUMBER DE SIDE DAYF FOINT DAN SOIL DUAK	A GIT	TABLE 6	NUMBER CF	BLO × 9 PE	150. 200. 250.			

2 42-10. DIAMETER PILES MLHB105FT 3-PILE STRUCTURES 200FT PENETRATION ** VULCAN 500 HAMMER

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N O
THICKNESS#1.50
1 1 ¥ 1.
DTIPE 420, MINIMUM FALL
450
0110

		K							
SEGHENT	ELEV	SLACK	MIN GH	Æ	COEF RSTITU	8 31			
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01	0.0	٥.	126,7	51,3	0	288512			i
	8.3	٥.	126,7	51,3	0	288512			
12	•	٥.	126,7	51,3	1,00	288512			
13	1.0	٩.	126,7	51,3	1,00	268512			
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17	8,3	٩.	126,7	51,3	0	288512			!
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19	1.0	9	274,9	21,2	0	417307			
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34	3.3	٥.	411.9	8.0	000	534713			
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36	•	٥.	772,6	8	1,00	188794			
37	æ æ	٩.	772,6	8	1,00	188794			
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63	2,5	٥.	772.6	. 3	0	188794			
			•						

2 42-IN, DIAMÉTER PILES MLWEIOSFT 3-PILE STRUCTURES 200FT PENETRATION ** VULCAN 560 HAMMER PROB

OTIPE,420,MINIMUM MALL THICKNESSHI,50 IN. RU # 50

TABLE 8 -- HAXIMUM STRESS DATA

TIP RESISTANCE PENCENTAGE # 50,00

PERMANENT SET OF PILE # .0362 INCHES NUMBER OF BLU4S PER FOOT # .331.12 TOTAL INTERVALS # 168

ELEV HAX C STRESS TIME N HAX T STRESS TIME N LES TOTALS	AN C STRESS TIME N HAN T STRES LUSSISSIN. LUSSISSIN.	TAX T GTRES	AX T STRES		TIME N	AREA SO. TO.	DMAX(M)	(H) Q]	(W) A
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0.00 15955. 4	955. 42	~	•		0	21,28	1,013252	75	9.5
50,00 14071, 44	071.	7	•	1	0	51,32	00700	1 3	1.06
21,67 14093, 4	093. 46	4	0		0	51,32	~	0	
15,53 14123, 49	123, 49	0	•		0	51,32	-	9	
55,00 14152, 5	152, 5		•	1	0	51,32	4	75	
6.67 14167, 5	167. 5		•		0	51,32	S	2	
8.55 14155	155	92	0		0	51,32	C	55	•
0.00 14026. S	0.26	•0	•			51,32	4	35	
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3,33 13751, 61	13751, 61	-	•		0	51,32	-3	35	-
5.00 [362]. 6	13621, 63	2	•	ĺ	-	51,32		2	0.1
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8,33 13507 ₆ 6	13507, 6	68 0.	•		0	51,32		96	80
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8 12012 03002	5075	50	0		0	21,28	Φ	.747503	67
20,23 13313, 8	מיוני	ָרָטָּ מאַ	.		0	21,28	S	.731870	09.
50.07 15.555. 8	15556	988	•		0	21,28	\sim	5	16
D	15100	000	•		C	21,28	m	.697612	. 88
V 00000 00000	4440e	0	•		0	21,28	÷	2	26
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70.00	17108	0	• 0	i	0	0.85	84	642159	96
6 986918	6918	0 66	•		0	0,85	3073	M	26.
16090, 10	6090, 10	01	•		0	0.85	61	782	
16427	6427. 10	0 70		(985	40	299	
(5,33 16133, 1	133, 10	0 90	•		0	0.85	6505	418	6.8
11.67 15779, 10	779 10	080	•		0	0.85	-	4	. 87
C.00 15410. 1	5410. 11	10	0	1	0	. 85	3112	126	98
23,89 14882, 11	4882, 11	12 0.	•0		0	0.85	1317	8943	.87
5/ "/A 14026, 11	4026. 11	13	•		0	0.85	596123	468260	60

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STRUCTURES SEG MAX T STRESS L88/80, IN. NO. 112888 110218 10218 7002 7002 7002 7002 7003 5043 7032 2003 1033 11353 11353 MLWA105FT 3-PILE 2 MAX C STRESS LBS/SQ.IN. NO. 200FT PENETRATION . VUICAN 560 HAMMER Z H 5950 6807 6875 6991 6916 OTIPE 420, MINIMUM MALL THICKNESSE1,50 20,00 RESISTANCE-BLUM CURVE DATA BLUESCYPT, REGISTANCE DYNAMIC PT TOTAL TIONS FURGETIONS 91.22 255.82 329.28 397.44 460.91 519.86 625 47 672 96 717 40 758 80 797 42 896 34 923 70 924 67 974 75 029 85 047 69 059 78 071 68 075 17 065 27 TIP RESISTANCE PERCENTAGE 97.24 120.80 156.22 215.36 • 50 67 58 32 99,00 80.47 8,89 • TABLE PROB

2 42-11, DIAMETER PILES MLMB105FT 3-PILE STRUCTURES 200FT PENETRATION -- VULCAN 560 HAMMER

RU # 50 DTIPH. 420. MINIMUM BALL THICKNESSHI, 50 IN.

TABLE 10 +- SPECIFIED BLUW DATA

20,00 TIP RESISTANCE PENCENTAGE .

Pile Driving Resistance Curves

Pile Diameter

Minimum Wall Thickness

Penetration

Hammer

Quake Factor, Tip

- 42 in.

- 1.50 in.

- 2.00 in. (Uniform)

- 250 ft.

- Vulcan 560

- .025 in.

- .10 in.

- .30 in.

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	EX/81 8.5.0	25/76.	80 80 80 80 80 80 80 80 80 80 80 80 80 8	38888 3888		33.00 39.00 39.00 39.00 39.00			
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0 0 9			>>>>	* * * * * * * * * * * * * * * * * * * *	>	> > > > > > > > > > > > > > > > > > >			E-0-12-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-
			**	>	ى ئى ئى ئ	11 11 11 11 11 11 11 11 11			

# SLACK WEIGHT AR (18) (18) (18) (18) (18) (18) (18) (18)	TABLE 1 == PROGRAM CONTROL DATA PILE TYPE REW MATERIAL DATA OPTION NEW MATERIAL DATA OPTION NEW SOIL DATA	
3 MATERIAL DATA WUMBER OF MATERIAL TYPES # 1 MATERIAL (TOD) UNIT WT MODULUS TYPE 1 42,000 490,0 2900000	3LACK WEIGHT AREA (IN) (LB) (80 IN)	SPRING CONSTANT (LG / IN) 6200000.00
(700) UNIT WT. (PCF)	3 ** MATERIAL DATA UMBER OF MATERIAL TYPES # 1	00.00
	(TDD) UNIT WT. (PCF) 42,000 499,00	

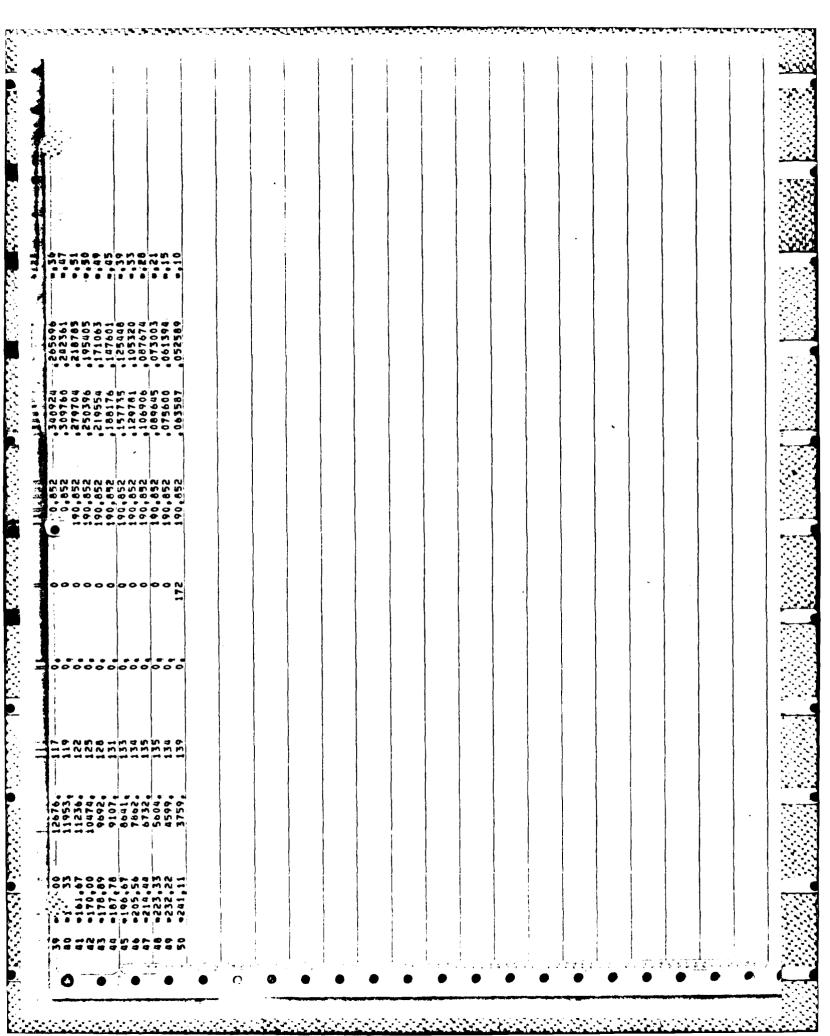
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PROPERTY SECURISES

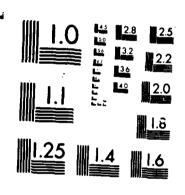
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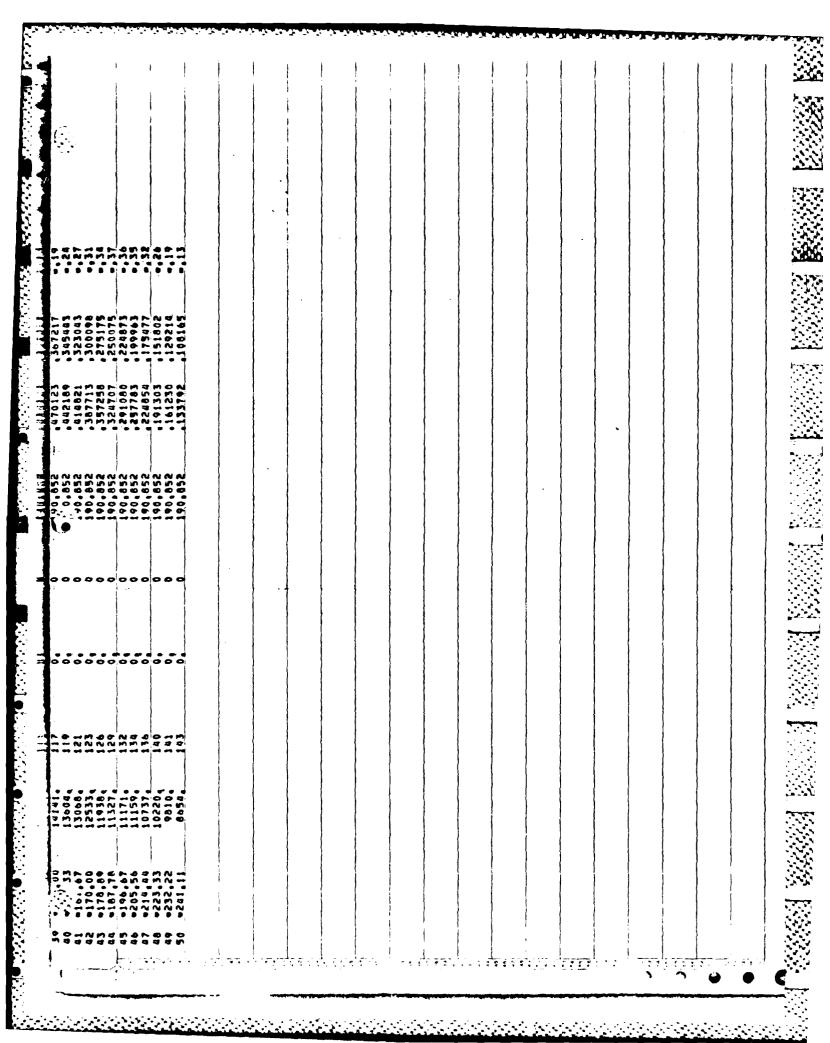


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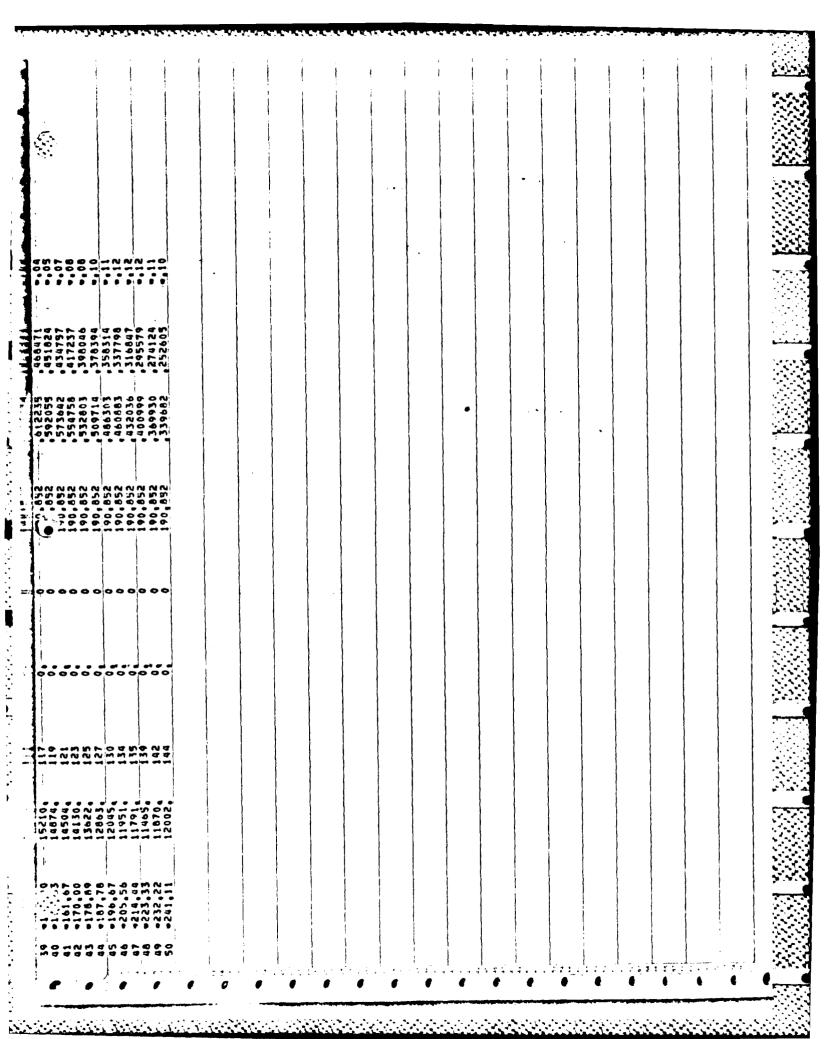
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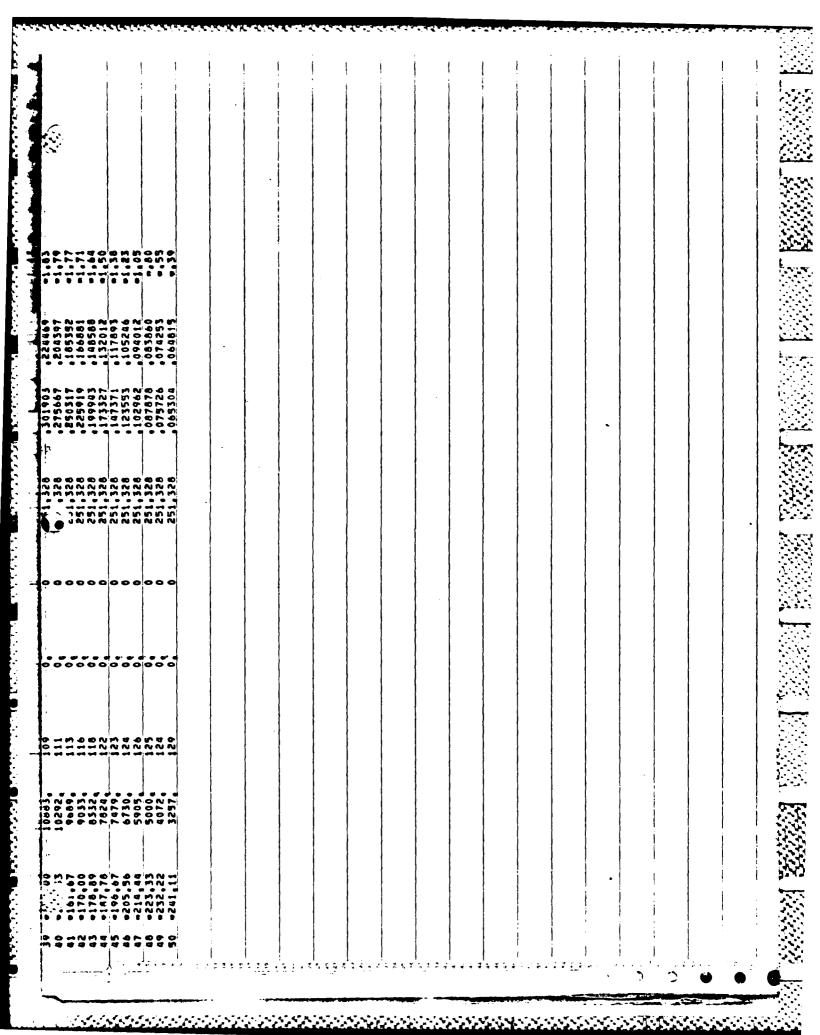
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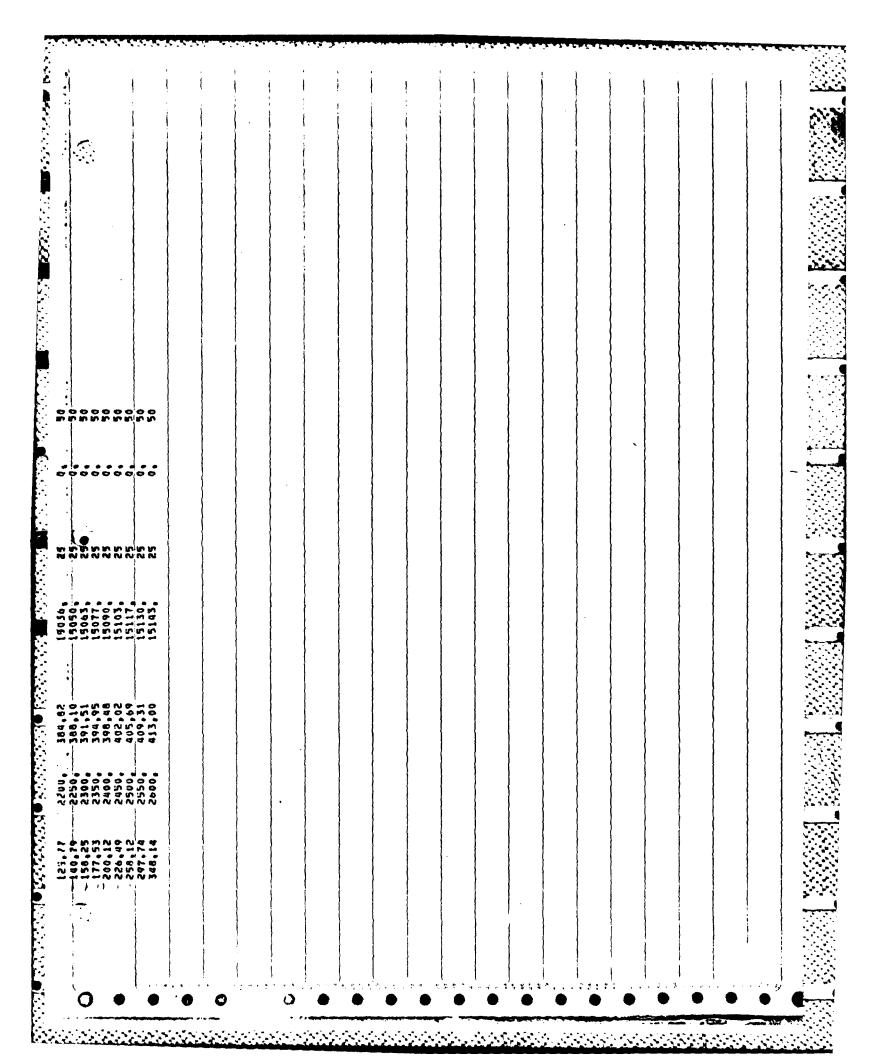
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PROB 250FT PENETRATION VULCAN 560 HA		
	42-IN. DIAMETER PILES MLWEIGSFT S-FILE STRUCTURES. VULCAN 560 HAMMER	
GTIPE,025, UNIFORM MALL THICKNESSEZ,0 IN.	#2.0 IN. RU # 14	
TABLE 10 SPECIFIED BLUM DATA		
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PRCB A2-IN DIAMETER HILES MLWBIOSFT 3-PILE ST 250FT PENETRATION PT VOLCAN 560 MAMMER GIIPEO.10. UNIFORM MALL THICKNESSE2.0 IN. TABLE 1 PROGRAM CONTROL DATA PILE TYPE AEN HAMMER DATA OPTION AEN HATERIAL DATA OPTION 1 AEN HATERIAL DATA OPTION 1	TION DITION E-BLOW CURVE T (FT)	DESCRIPTION ENERGY ENERGY ENCLOSIVE FORCE (LOS) OF HARMER OSGMENTO T SLACK MEIGHT AR T SLACK MEIGHT AR	1000,00 60000,00 1000,00 42000,00 RIAL DATA MATERIAL TYPES . 1	(TCD) UNIT WT. (PCF)

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NUMBER OF TIP RESISTANCE PERCENTAGES NUMBER OF TIP RESISTANCE - 15 POINT DAMPENING RESISTANCE - 15 SOIL OUAKE FOR SIDE - 10 SOIL OUAKE FOR POINT - 15 SOIL OUAKE FOR POINT - 10 SOIL OUAKE FOR POINT - 10 TIP RESISTANCE - 10 TABLE 6 == SPECIFIED BLUM COUNT OATA NUMBER OF SPECIFIED BLUM COUNTS 4 SOO			1.500	909	330	4:0		
NUMBER OF TIP RESISTANCE = 150 (15 street) SOUTH DAMPENING RESISTANCE = 1500 (15 street) SOUTH DAMPENING RESISTANCE = 1500 (15 street) SOUTH DAMPENING RESISTANCE = 1500 (15 street) SOUTH DAMPENING RESISTANCE = 1500 (15 street) TIP RESISTANCE = 1500 (15 street) TABLE & == 5PECIFIED BLUM COUNTS & 4 street) BALDAS PER FOR STREET STR	TABLE 5	301L						
POINT DAMPENING RESISTANCE * JPONT 19 SOIL GUAKE FOR SIDE * 6 SIDE * 10 BOINT			MANCE PERCENTAGES	-				
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	SOFT PENETRATION	N VULCAN 560	60 HAMER		TATION		
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TABLE 9 00	* RESISTANCE -BLUM	CURVE	DATA				
416	RESISTANCE P	PERCENTAGE &	35,00				
8L0H8/FT	RESISTANCE TOTAL-TONS	PURCE-TONS	MAX C BTRESS LBG/80,1N, NO.	BEG HAX	TAX T STRESS	0 1 5 1 5 1 5 1 5 1 5 1 5 1 5 1 5 1 5 1	- }
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2080	250FT PENETRATION ** VULCAN 560 MAMMER	
	D.10. UNIFORM MALL THICKNESS:	
	CIFIED BLUW DATA	
	TIP RESISTANCE PERCENTAGE # 35.00	
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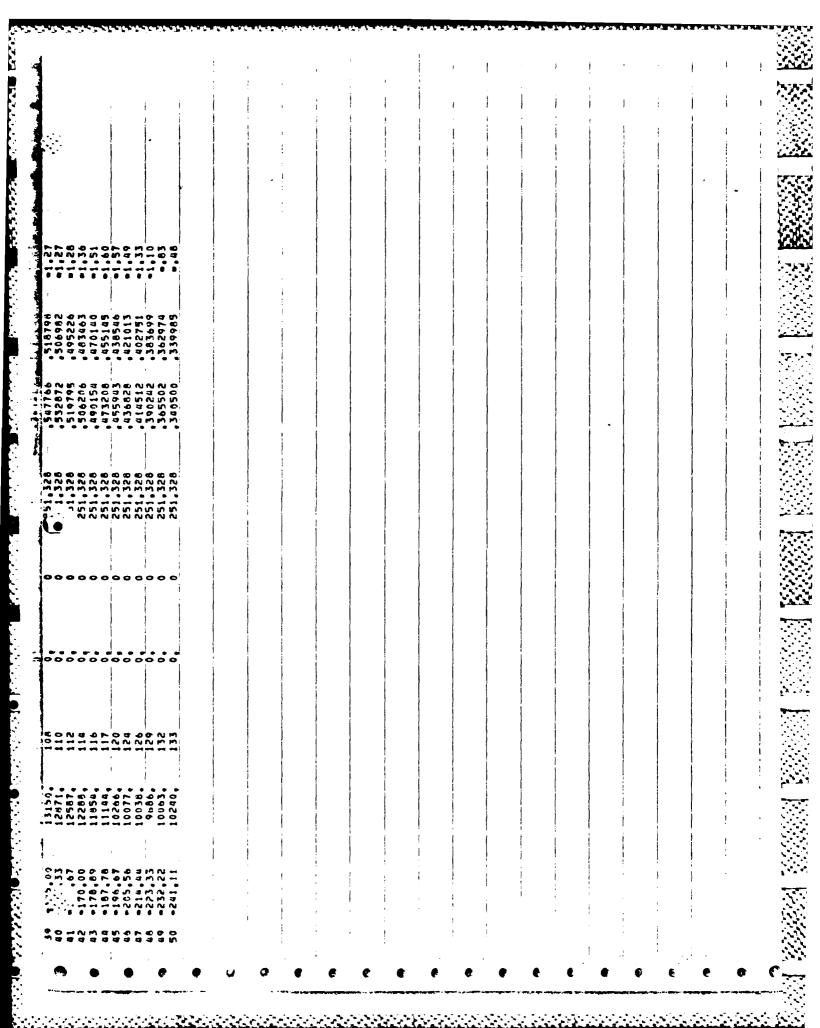
PROB 42-IN, D 250FT PE OT 1PB-50 TPB-	JUNE 1976 JUNE 1976 108 4 25.0FT PENETRATION VULCAN 560 HAMMER GTIPH, DIAMETER FILES MLWHIGSPT 3-PILE BTRUCTURES GTIPH, 50. UNIFORM WALL THICKNESSHZ, 0 IN, RU # 50 SLE 1 4- PROGRAM CONTROL DATA PILE TYPE NEW HAWER DATA CPTION 1
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NOTARA ENERGY NO	ENERGY (FIGURE) 100000,00 EXPLOSIVE FURCE (LBB) 00,00 OF HAMMER BEGMENTS 2 T BLACK WEIGHT AREA COEF OF BPRING CONSTANT (LB) (SG IN) RESTITUTION (LB / IN)
1 2 5	000,00 6000,00 000,00 42000,00 DATA
TABLE 4 00 PILE 8E	42,000 490,0 29000000, 8ECTION DATA

STATION NUMBER TOP BOTTOM LENGTH 160,00 TABLE 6 -- SPECIFIED BLUM COUNT DATA MATERIAL MALL THICKNESS TYPE (IN) NUMBER OF SPECIFIED BLUM COUNTS 22.52 TOLERANCE POINT DAMPENING RESISTANCE BOIL GUAKE FOR SIDE SOLL GUAKE FOR POINT TIP HESISTANCE PERCENTAGE 50,0000 TABLE 5 -- BOIL DATA BLOWS PER 250. 250. SECTION NUMBER

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	WHIOSPT Suplic	# 50				AREA SO. IN.	1,00	251,328	51.5	51,32	51 32 51 32	51, 32 51, 32 51, 32	51,32	51,32	51.57	51,32	51,32 51,32	51,32	51,32 51,32	51,32	51,32	51,32 51,32	51,32	51,32	51,32
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6 42mIN, DIAMETER PILES HLWB105FT 3mPILE STRUCTURES PROS

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TABLE 9 44 RESISTANCEPRION CORVE DATA

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		BLO*S PER FUGT	REGIGTANCE	
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TO SECURE AND A SECURE AND A SECURE ASSESSMENT

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Pile Driving Resistance Curves

Pile Diameter - 36 in.

Minimum Wall Thickness - 2.00 in. (Uniform)

Penetration - 250 ft.

- 200 ft.

Hammer - Vulcan 060

Quake Factor, Tip - .025 in.

- .10 in.

- .30 in.

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SPRING CONSTANT 00000026 MAYE EQUATION ANALYSIS FOR 36-IN. DIAMETER PIPE PILES
WC CILLLAND REPORT DATA FUR 1CHR 3-PILE STRUCTURE -- BURING
WAY 25,1976 50 0 50 0 50 0 50 0 AREA COEF OF SA IN) RESTITUTION 00 VULCAN DED MAMMER SOUTH OF DETER PILES SAPILE STRUCTURE 250FT PRAFTRATION ** VULCAN OGG HAMMER OTTPE,025, MINIMUM MALL THICKNESSEZ.O IN. HOPULUS (PSI) 840,0 29000000 ULTINATE RESISTANCE INCHEMENT (TONS)
MAX ALONS FOR DESISTANCE ARION CURVE (BPF) 00 CNIT MT (PCF) 600000,00 1000,00 42000,00 RETCHT (LH) SPECIFIED SECTENT LANGUE (FIL) NEW MATERIAL DATA OPTION NEW MATERIAL DATA OPTION NEW PILE SFCTION DATA OPTION DETAUT OPTION FING STRESS AND ROPE FOR STRESS NUTBILL INPTION MAKER ENERGY (FT-LAG)
MATER EXPLOSIVE FORCE (LAG)
NUMBER OF MAMPER SEGMENTS SPECIFIED ALTA COUNT OPTION NUMBER OF MATERIAL TYPES & TABLE 1 .- PROGPAM CONTROL DATA 36.000 (100) TAMIE & .. PILE SECTION DATA DATA NOTION 000000 TABLE S .. HATERIAL DATA WALVER DESCRIPTION TABLE 2 -- HAMMER DATA SLACK 3 MATERIAL SECVENT RUMPER Q 47 2 4 1 8 C 8 G

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TIP RESISTANCE 14,0000 TABLE 6 SPECIFIED BLOW COUNT DATA ALTHER OF SPECIFIED BLOW COUNTS 44,000 250 250 250 250 250 250 250
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PERFORM RET OF PILE B		1	60	1	£						
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FILE V MAK C STRESS TIME N WALT STRESS TIME N AREA DWAKEPY TO (PH) VIPE N WALT STRESS TIME N SO, MAY TO 1, 22, 293 1, 32, 293 1, 32, 294 1, 32, 324 1, 32, 32, 32, 324 1, 32, 32, 32, 32, 32, 32, 32, 32, 32, 32				611	a 122						
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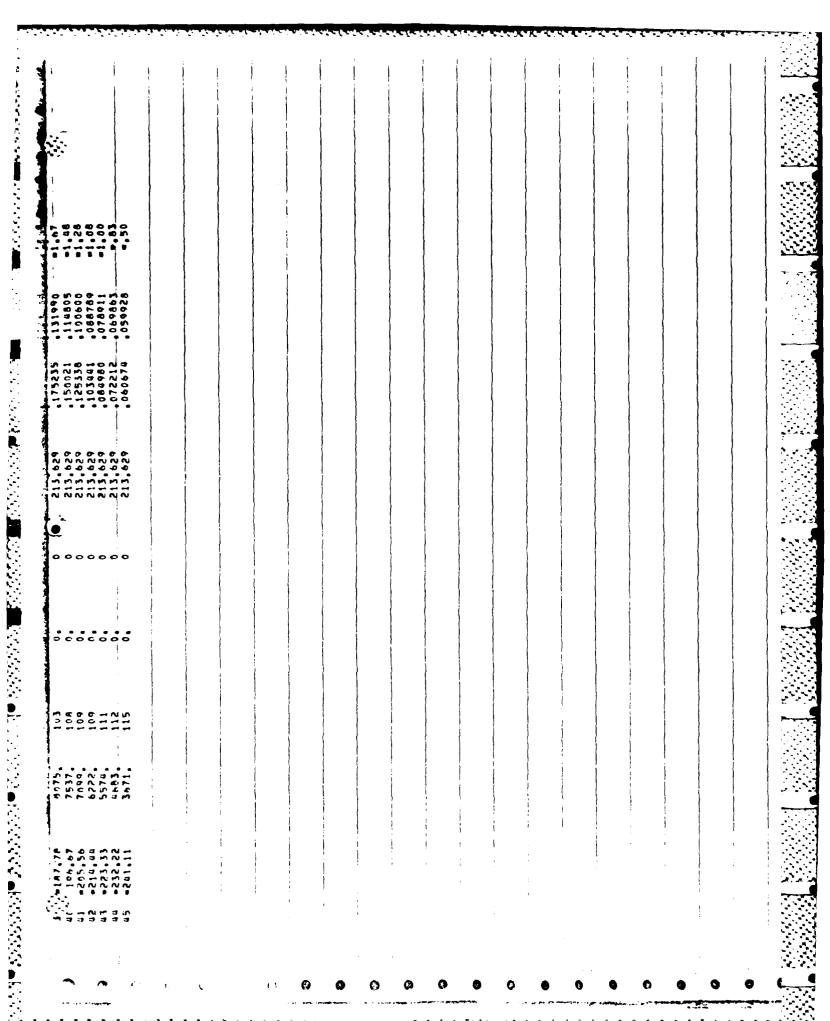
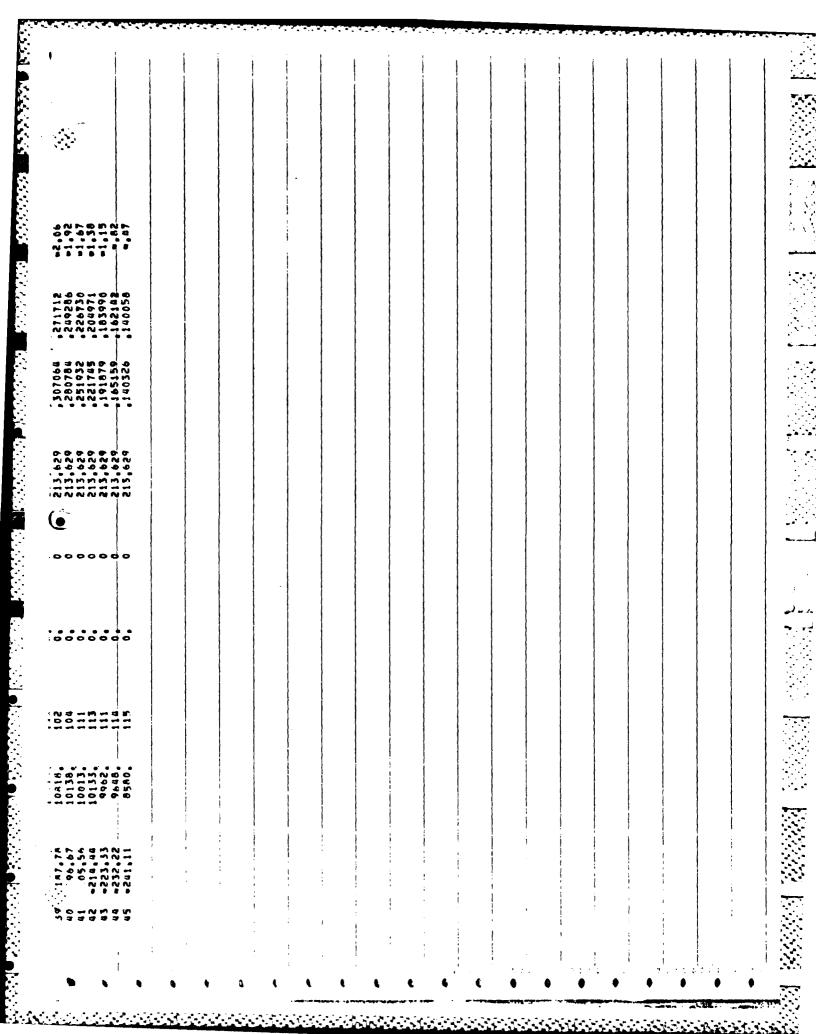


TABLE	250FT	PENETRATICN	36-IN. DI	AMETER PILES 34	*PILE STA	STRUCTURE		
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250FT PENETRATION OTIPE,025, MINIMUM WA TABLE 10 SPECIFIED BLO TIP RESISTANCE PERCE BLOWS PER 138,93	BU B	38	
OTIPE, 025, MINIMUM WALL THE 10 SPECIFIED BLOW DATA 11P RESISTANCE PERCENTAGE BLOWS PER 138,93	14 NC E		
10 SPECIFIED BLOW DATA TIP RESISTANCE PERCENTAGE BLOWS PER R FOOT			
RESISTANCE PERCENTAGE BLOWS PER FOOT 138,93	< 0; → N M ⊃		
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295.03			
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	2 36+IN. 250FT PENETRATION AN VULCAN	36-IN, DIAMETER PILES S-PILE STRUCTUR VULCAN 060 HAMMER	
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SPRING CONSTANT **\$200000.00** CLB / INJ MAVE EGUATION ANALYSIS FOR 36-IN. DIAMETER PIPE PILES 60.00 AREA COEF OF (SQ IN) RESTITUTION 966 VULCAN DED HAMMER Shein, Diaufiff biles Sebile structure 2007 Fenfication as Vilcan OSO HAPMER STIPE, 025, 41N IMUY BALL THICKNESSEZ, O IN. 490.0.29000000 (100) LINI HIS FOUNTED HPP FOR STRESS CUTPUT OPTION.
ULTIMETE RESISTANCE INCREMENT (TONS)
MAX BLOWS FOR PESISTANCE BLOW CURVE (BPP)
SPECIFIED SEGMENT LENGTH (FT) 1.00 30,00000 42000,00 RFIGHT TABLE 1 -- PRUGRAM CONTROL DATA (87) NEW PILE SECTION DATA OPTION NEW SOIL DATA OPTION HANNED FIXELDSIVE FORCE (LASS) SPECIFIED ALLON CIVILLY METIGN NUTROLT OPTION FOR STRESS NUMBER OF MATERIAL TYPES R STREET OF TAXABLE SERVICE 36.090 NEA MATERIAL DATA MATION TARLE & oo PILE SECTION DATA NEW PAYPER DATA OPTICA MANER ELERGY (FT-LESS 1000,00 TABLE 3 .. MATERIAL DATA SLACK KIN) .. MANYER DESCRIPTION MANYER EFFICIENCY TABLE 2 .. HAMPER DATA MATERIAL OBBACK BEGFFNT MAY 25.1976 PRCB 3

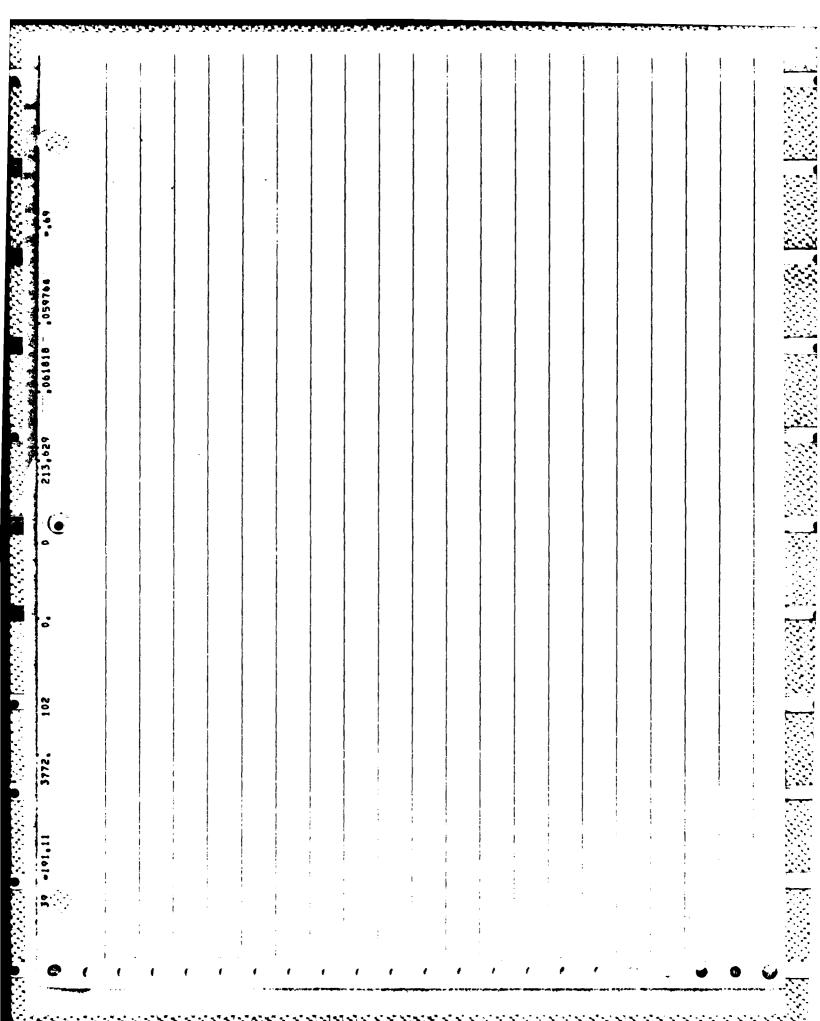
TABLE 6 SPECIFIED BLUM COUNTS A NUMBER UF SPECIFIED BLUM COUNTS A RICHS DER FOOT 250 250 250 250 255 300, 255
TIP RESISTANCE TABLE 6 SPECIFIE RUNAER OF SPECIFIE FOOT 250 250 350 350

STATEMENT PROGRESSION

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	6	103,3	្ម	6057.76	<u>r</u> :	1.00	61952352	
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	36	7.7	٠,	461	2	1.00	80330	
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	38	-182,22	0.0	461.	2	1.00	33	
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; ;	OTIP4,025,4I	HININDH MALL T	THICKNESSEN	a .vi o.	U = 14				
TAB	ILE B == MAXIVUM	M STRESS DAT	4						
:	TIP REGISTAN	CE PERCENTAG	E = 14	00.					
; ;	PERMANENT GET NUMBER OF MICH	SET OF PILE A ALOWS BER FOOT RVALS		INCHES					
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36-1N. DI VULCAN 0	H WALL THICKNE	ANCE-BLOW CURVE D	PERCENTAGE 8	DYNAMIC PT FORCE-TONS	4		95.83	3.0			7.	· c	2	1 0	. E		3.0		್ತಿ	~. -					- 4		~,"				3.					
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8084	200FT PENETRATION	SG-IN, DIAMETER P. VULCAN GGO WAMME!	36-IN, DIAMETER PILES 3-PILE STRUCTURE 		
	OTIPE.025, MINIMUM WALL THICKNESSEZ.O	LL THICKNESSEZ.O	IN. RUB 16	··	
748	TABLE 10 SPECIFIED BLOW DATA	H DATA			
	TIP PESISTANCE PERCENTAGE &	NTAGE . 14,00			
	BLOWS PER	RESISTANCE			
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	252,32	2450.			
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SPRING CONSTANT 05000026 - (MT / BT) -MAVE EQUATION ANALYSIS FOR 36-IN, DIAMETER PIPE PILES
PC CLELLAND REPURT DATA FOR ACMR SAPILE STRUCTURE -- RORING 1
MAY 25,1976 RU E 39 AREA COEF OF COE IN THE STITUTION 35 VULCAN DEN MAFFR SOUTH OF STREET PILES SOUTH STRUCTURE SCORT PRINTATION OF VUICAN GOO MANNER STIPESTO MANNER ST POD JUS THE STEESS IN THAIT DOLLOW CONST. (1904)

OF THEST REGISTANCE INCREMENT (1904)

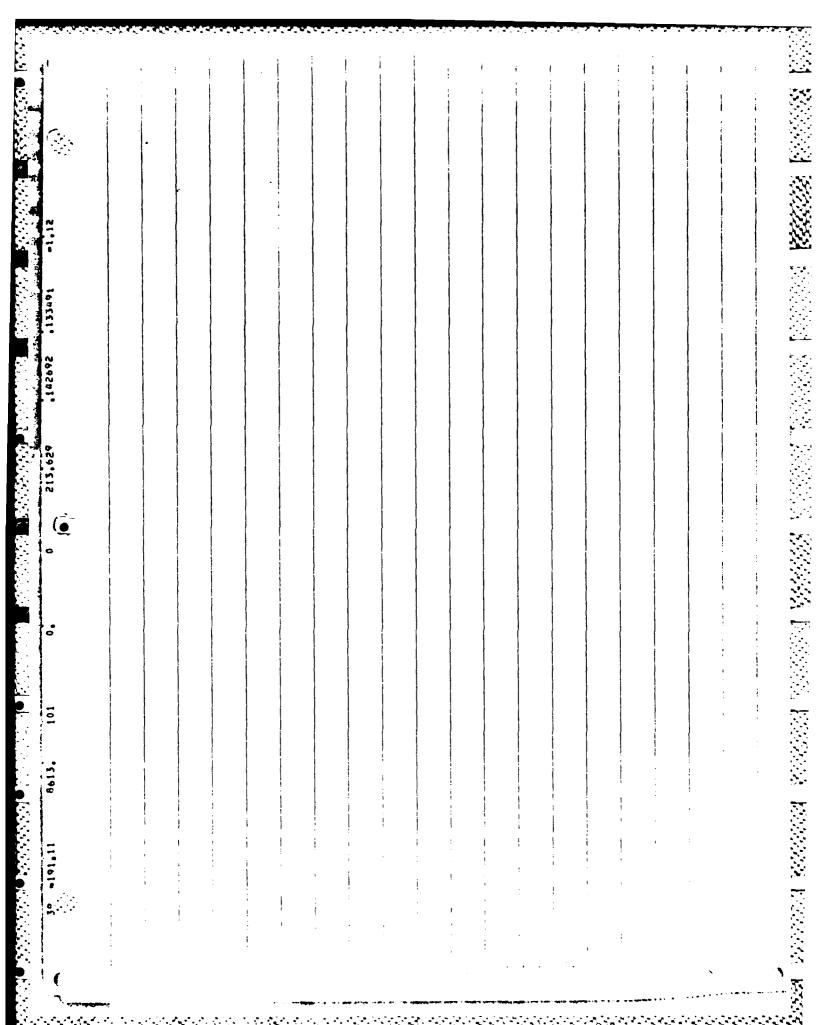
FAX BIDAS FOR HESTSANCE HENDE (MPF)

OFFICIALES GROKELT [MAI] 1.00 3000000 UNIT MI (656) 60000.00 "EIGHT (63) SECTION DATA OPTION SPECIAL BARROLIGHE GRANET NUMBER OF HATERIAL TYPES,R .. NE* SOIL DATA CRITON SPECIFIED ALOW COUNT CRITON TABLE 1 -- PRUGRAM CONTROL DATA 36.000 MATERIAL DATA SPITCH CUTPUT OPTION FOR STRESS TABLE 4 -- PILE SECTION DATA_ (100) TALLER OF SCHIBSTON
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TA P DATA CPTICA TABLE 3 -- MATEPIAL DATA 1600,00 SLACK TAPLE 2 OF HAMPER DATA MATERIAL トアルコロルの 医多数人的人 720 5

STATION NUMBER TOP BOTTOM 2200 LENGTH (FT) 120,00 TABLE 6 -- SPECIFIED BLOW COUNT DATA MATERIAL MALL THICKNESS TYPE (IN) NUMBER OF SECTIONS CHANGED NUMBER OF SECTIONS ADDED LENGTH OF FREE STANDING PILE(FT) NUMBER OF SPECIFIED BLOW COUNTS 22.52 TOLFRANCE TIP PESISTANCE PERCENTAGE 35.0000 TABLE S -- SOIL DATA 150. 200. 250. BLOWS PER SECTION NUMBER

edito						
	. 10 , MINIHUM	WALL TH	ICKNESSAP.0	IN. RU #	1 35	
TARLE 7	PILE SEGMENT	ENT DATA				
SEGMENT	ELEV	SLACK	MESGHT	AREA	COEF RSTITU	or
	L .	Z	9 6 7	. Z		au/11
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^ :	0.00	9 '	6742,58	2	00	57565269
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۰.) C	•	45.00 AB		•	
۰ ~	20	3 9	6545 TA			3/305034 5/46/4080
			A542 18		•	5714128G
• •	•	•	25.00 LB		-	2-202004 Retarand
· <u>-</u>		2	5547 AR	, v	- E	3/303034
			A5.62 TA	4	•	RTARDEO
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7	2 4	•	6087 74	0 T		4.06.946.9
	2		6057.76			61 - 25 - 25 - 6 61 - 65 - 25 - 6
	0	9	6057.76	13.6	1.00	61952552
-		0	6057.76	13.6	1.00	61952352
£.	۰	٥.	6057,76	13.6	1000	61952352
19	9	٥.	6057,78	13.6	1.00	61952552
20	~	٩.	6057,76	13,6	00.1	61952352.
~	٠	e.	6057.76	13.6	1,00	61952352.
- 22	ç	੍ਹੇ	6057.76	, P	7.00	61952352
5 2	~	٥.	6057,76	3.6	•	61952352.
2 4	61.0	ç	6057.76	2.	000	61952352
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Š			6087.75			61 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 -
56	103.3	9	6057.76	3.6	•	61952352
08	11.6	0	6057.76	13.6		61952352
31	120.0	0	6461.61	13.6		56080330
32	8	٠.	6441.61	13.6	100	58080330
33	7.7	°.	6461,61	13.6		58080330
34	6.6	0	6461.61	13,6	•	58080330
35	5.5	٠	6461.61	13,6	1.00	58080330.
36	90,0	00.0	6461,61	13.6	1.00	58060330.
37	73.3	ç	6461.61	13.6	•	58080330.
36	2,2	0	6461.61	13.6	1.00	58080330.
0	•	C	14.144	* * *		E2020110

50 64 64	20051 PENETRA	5 36=10 A710w == YUL	N. DIAMETER PIL CAN OGO MAMER	ES Sepile	STRUCTUR				
	OTIPE, 10 , MIN	NIHUH WALL T	HICKNESSEZ.O IN	3 %	35			·	
TAB	BLE S MAXINUM	STRESS DAT	A						
	TIP RESISTANC	E PERCENTAG	E . 35,00			,			
	PERMANENT SET NUMBER OF BLC TOTAL INTERV	TOF PILE OWS_PER_FOOT ALS	8 2027 INCHE	£3					
ELEV	MAX C STAFESS	Z HIE S	HAX T GTREGG	2 85 56	AREA	DHAX(H)	DCM	V(H) 178EC	
•	912798.	30		L 1	ိ	1997	60		
0	166	26		0		0000	20 K Q.		
. O	0 G	B 00	• •	00	~~~	8 to 60	5 P P P	•	
C (700	33			13.6	5305	275	~;	
0	516	64 74	• •		13.6	2194	250	Ň	
C (\$ P	30			13.6	9420	755	•	
9	r. w	t t	••		13.6	, 2	353	1,34	
	519	000			13.6	4537	9170	'n.	
9 0	•				13.6	1228	9	iv	
& W	770	52			4.4	8446	7426		
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30	336	65			13.6	8176	200	7	
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0	401	2			13.6	5077	654	'n	
M 4	C 4	77			13.6	52267	417	·.	
90	219	91			13.6	6616	921		
103.3	900	83	•		5	3798	999	2	
120.0	- "	8 eo	• •		13.62	104E	417 170	20.10	
178.A	3 2	00			13.62	5542	0	2	
137.7 146.6	⊶ ru	ው ው የነ ኒሳ	• •		7. 7.	54 590	657 416		
155.5	102	66	•	İ	13.62	6289	197	9	
e173.53	10466	000	• •		215,629	196685	196400	1 1 2 4 0 4 0 4 0 4 0 4 0 4 0 4 0 4 0 4 0 4	
		•	•		•				



8084	OOFT PENE	S ETRATION	36-1N. D	IAMETER PILES SO DOO HAMMER	SepilE ST	STRUCTUR		
10	11Pm, 10	HOMINING	WALL THICK	NESSEZ.O IN.	RU = 35			
TABLE 9	. RESI	STANCE	LOW CURVE	DATA				
II.	PRESIS	TANCE PE	ERCENTAGE .	35,00				
/SM019	FT. RES	19TANCE	DYNAMIC PT	MAX C STRESS LAS/SQ.IN. NO.	SEG LAS	MAX T STRESS	918	•
-	಼್	•	5.	16477.	12	14435.	•	
2		0	200	16589	25	13519	0	
~ .	ີ້	000	77.0	16671	2 2		•	
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• I	•	6	35.4	16903	2	9587	-	
G	.	200	\$ * * * *	91691	0 0	3000	~ •	
21	: •	9	50,3	16943	20	6751	9	
	۰.	0	96	16956	O N	8009	• •	
14	•	50	31.2	16969	2	5419		
~ (c. c	5 6	63.6	16982	<u>ې</u>	4769	•••	
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50	٠.	0	72.3	17033	8	1475	32	
25 22	^	50	95.3	17045	20	940	32	
2	æ 1	6	16.8	17058	Ĉ.	508	32	
25 75	^. >	7 C	9	17070	2	N. C.	25	
50	. =	0	72.6	17095.	0		20	
31	٠.	100	88.6	17108.	0	•	30	
75	~	150	35.5	17121	2	0	39	
ŠE .	~	0	17.2	17134,	0		39	
٠, .	٠,	20	29.6	17147	<u>.</u>	•	6 0	
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19	~	00	80.0	17211.	5	0	39	
.e	٠,	20	87.8	17224.	10	•	39	
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	47		18.6	17299		• •		
901		6	20.3	17312,	6	0	30	
•	70	950	20.3	17324,	61	•	29	
727	. 15.	2000	919,55	17337	<u>e</u> (•	6 00	
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•	•	>		(107)				

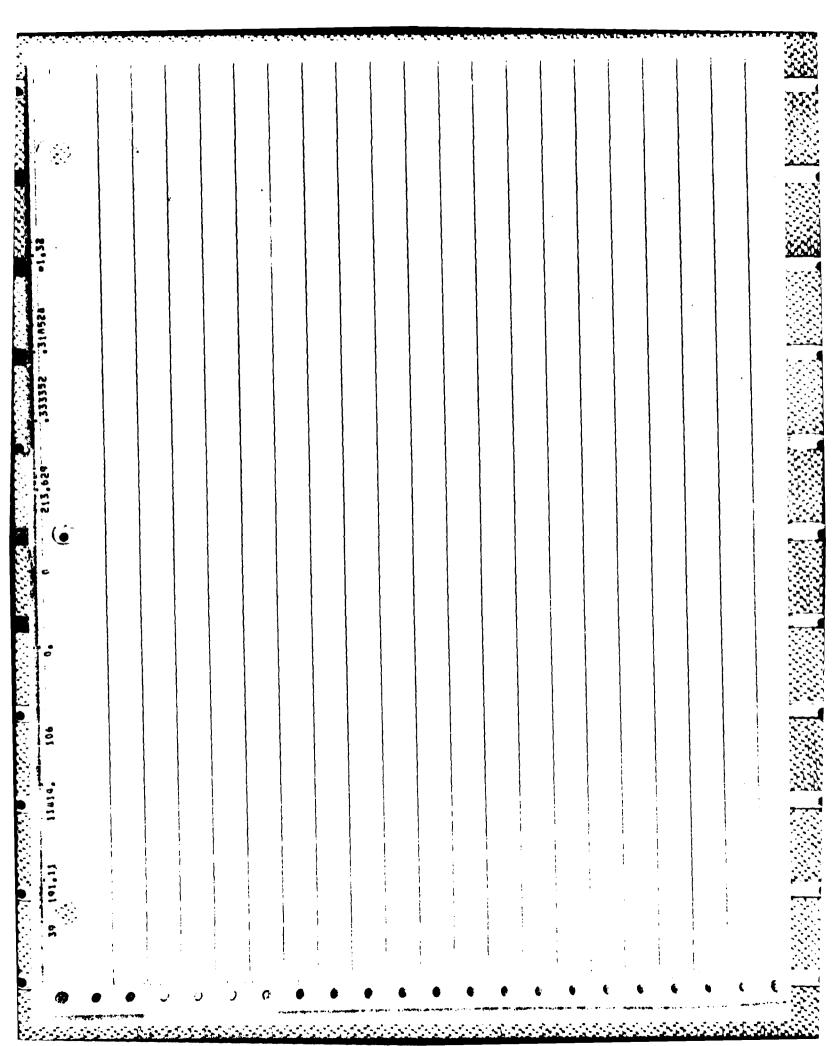
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TABLE 10 -			Killer Coo N	-,	
	E.10 , WINIMUM WAL	OTIPE,10 , MINIMUM WALL THICKNESSEZ,0 IN.	RU = 35		
	TABLE 10 SPECIFIED BLOW DATA	DATA			
	TIP RESISTANCE PEPCENTAGE	TAGE # 35.00			
	BLOWS PER FOOT	REGIGTANCE			
	137,07	1050.			
	227,37	2000.			
	; ;				
11 4 4 7 7 7 1					
•					
† •					

SPRING CONSTANT 9200000,00 CLA / 141) WAVE EQUATION AMALYSIS FOR BOWING DIAMETER PIPE PILES MC CLELLAND REPORT DATA FOR ACHR SOPILE STRUCTURE of BORING MAY 25,1976 50°0 (SO IN) PESTITUTION COEF OF VULCAN DED HAMMER SOUT PENETRATION ** VUICAN DAD HANNER OTIPE, SO MANNER OTIPE, SO , AINIMUM MALL THICKNESSEZ.O.IN. 490.0000005_0409b MUDUL US (P81) ULTIMATE RESISTANCE INCREMENT (TONS)
MAX RICHS FOR RESISTANCE-SLOW CURVE (SPR)
SPECIFIED SEGMENT [ENGTH (FT) 00.0 200000 00 UNIT WT (PCF) 00.00000 42000.00 HE IGHT NEW MARKER DATA OPTION
NEW MATERIAL DATA OPTION
NEW PILE SECTION DATA OPTION
NEW SOIL DATA OPTION (LA) APF FOR STRESS OUTPUT CIPTION HAMMER EXPLOSIVE FORCE (188) SPECIFIED ALOW COUNT OPTION TABLE 1 -- PHIGBAM CHATROL DATA. CUMBER OF MATERIAL TYBES & 26,000 TUMBER OF MAMMER SEGMENTS AUTPUT DPITON FOR STRESS TABLE & -- PILE SECTION DATA MAKKED FREDGY (FT-LAS) 1000.00 1000.00 TAMLE 3 -- MATERIAL DATA HANFER DESCRIPTION SLACK (IN) TABLE 2 .- HAMMER DATA HAMMER EFFTCIENCY MATERIAL 7.70尼 BEGLENT KUMBER 9 B C B •

STATION NUMBER TOP BOTTOM 190 240 320 LENGTH (FT) 120,00 TABLE 6 -- SPECIFIED BLOW COUNT DATA NUMBER OF TIP RESISTANCE PERCENTAGE SIDE DAMPENING RESISTANCE + JSIDE POINT DAMPENING RESISTANCE + JPONT SULL QUAKE FOR SIDE POINT POINT ... MALL_THICKNESS. (IN) NUMBER OF SECTIONS ADDED LENGTH OF FREE STANDING PILECETS NUMBER OF SPECIFIED BLUM COUNTS 2000.2 ะรู้รู้รู้ TOLERANCE .. TIP RESISTANCE. NUMBER TYPE 50.000 TABLE_5....301L.DATA BLOWS DER 200. 250. 300. The second second

EV LEASTANCE PERCENTAGE0134 LUCHES JUPILE STRUCTURE CONTINUED WALL THIERWEERS OF0134 LUCHES	Corr Pereralin Sarity Directs Peris Darlie Structure					•	• .				
TABLE 8 MAXIVUM STRESS DATA TIP RESISTANCE PERCENTAGE 4 50.00 BERNANET SFT OF PILE 8 .0334 IVCHES WHATE DEFENTAGE 4 .0334 IVCHES WHATE DEFENTAGE 6 .0034 IVCHES WHATE DEFENTAGE 7 .0334 IVCHES WHATE DEFENTAGE 7 .0334 IVCHES WHATE DEFENTAGE 7 .0334 IVCHES WHATE DEFENTAGE 7 .0334 IVCHES WHATE DEFENTAGE 7 .0334 IVCHES WHATE DEFENTAGE 7 .0334 IVCHES WHATE DEFENTAGE 7 .0334 IVCHES WHATE DEFENTAGE 7 .0334 IVCHES WHATE DEFENTAGE 7 .0334 IVCHES WHATE DEFENTAGE 7 .034 IVCHES WHATE DEF	TABLE B =	PRU9	OOFT PENET	TAON	STAMETER OBO HANN	ILES 3.PI	9		-		•
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The Resistance Percentage at 1 1 1 1 1 1 1 1 1	TIT RESISTANCE PRECENTAGE 550.00 TITLE	1841	E S HAXI	STRESS D							
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45.67 12953, 92 0. 0. 0. 213.629 . 6510.93 . 6520 . 6520 . 6520 . 6520	23.55 11735 93 0.0 0 213.629 .061973 .076697 8.55.59 11735 93 0.04220 55.59 11735 93 0.0 0 213.629 .015908 .355662 73.53 11155 101 0.0 0 713.629 73.53 11155 101 0.0 0 713.629	SS	1591			0	3,62	2000	4562	10010	
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200FT	PENETRATION	· VULCAN	DIAMETER SOLUTION OF THE SOLUT		# D L L D D M L D M L D	;
OTIPE	HUMINIM" OE"	WALL THE	CKNESSEZ O IN.	8 30 8 30		
TABLE 9	RESISTANCE-BLOW	CURVE	DATA			
- 11P. RE	RESISTANCE_P	PERCENIAGE B.	50,00			
BLOWS/FT.	RESTSTANCE TOTAL - TONS	DYNAMIC PT FORCE-TONS	MAX C STRESS LBS/SD, IN-NO.	SEG	/SO.IN. NO.	9EG
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	4504	3.	16924	21	6364	7
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3	160	Ö	17146	0	0	30
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Q		36-IN. DIAMETER PILES 3-PILE STRU	מדמטכזטא	
	STIPE, SO , MINIMUM MALL THICKN	THICKNESSEZ, O IN. RU # 50		
	TABLE 10 ** SPECIFIED BLOW DATA	DATA AGE # 50.00		
	BICAC PER	TOL		•
	126.34 195.14 255.28 291.95	14504 15504 16004 1621.		
;				
				23.00.03

Pile Driving Resistance Curves

Pile Diameter

Minimum Wall Thickness

Penetration

Hammer

Quake Factor, tip

- 42 in.

- 1.75 in. (Uniform)

- 250 ft.

- 200 ft.

- Vulcan 060

- .025 in.

- .10 in.

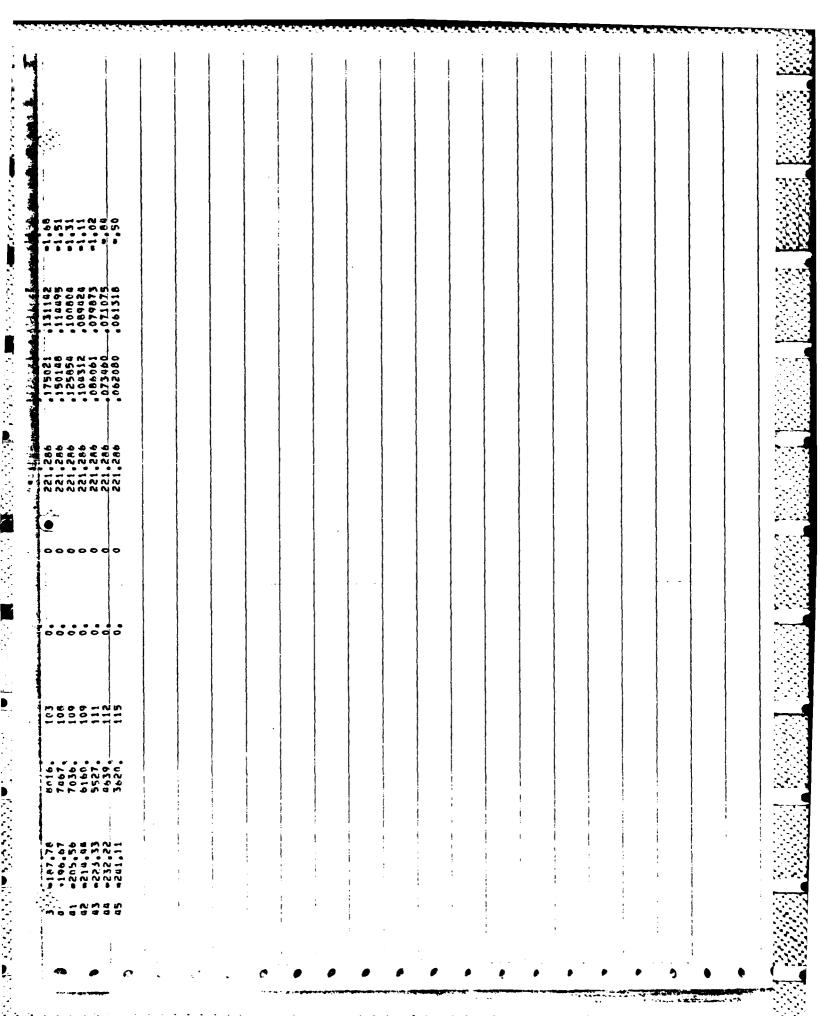
- .30 in.

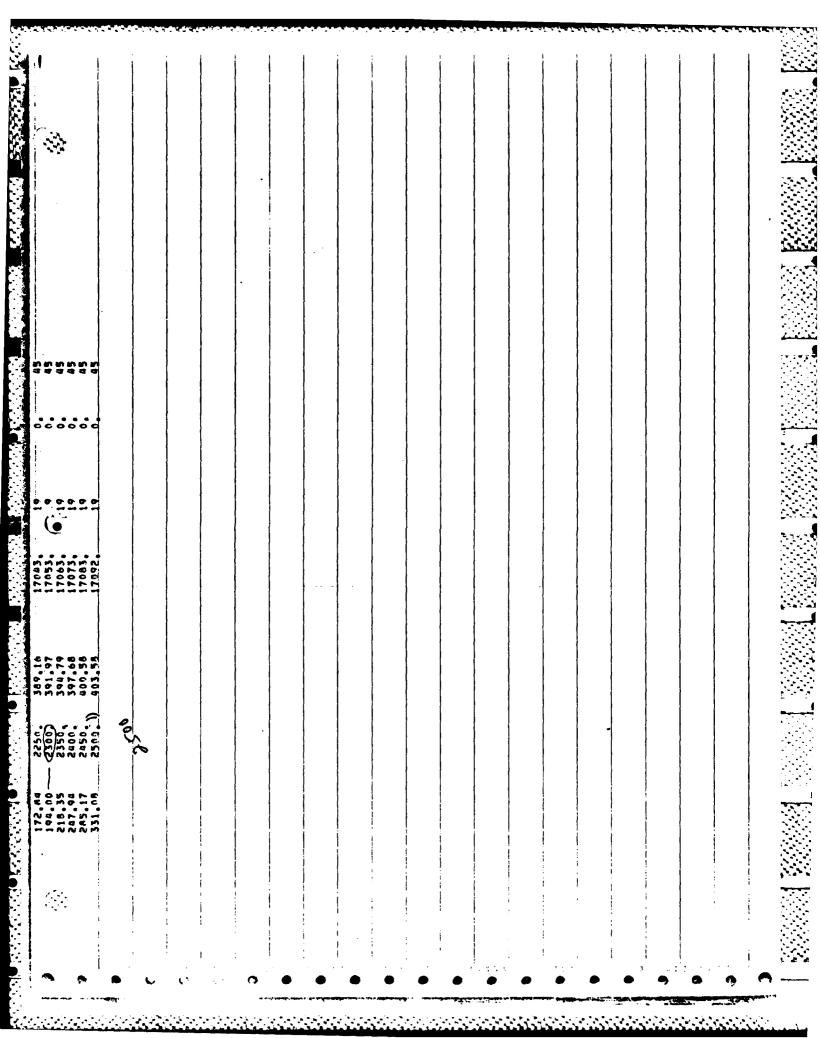
SPRING CONSTANT 92000026 CLB / IN) MAVE EDUATION ANALYSIS FOR 42-IN, DIAMETER PIPE PILES
MC CLELLAND REPORT DATA FOR ACHR 3-PILE STRUCTURE -- BORING
MAY 25,1976 AREA COEF OF (SQ IN) RESTITUTION .000 5000 000 VULCAN DED HAPMER ASSET DENETER PILES SAPILE STRUCTURE SAST DENETRATION OF VUICAN OGG HAMMER DTIPS,025,4INIMUM MALL THICKNESSEL ZSIN, MODULUS (PSI) 400,0 29000000 NEW MARWER DATA OPTION
NEW MATERIAL DATA OPTION
NEW PILE SECTION DATA CPTION
1
NEW SOIL PATA OPTION
SPECIFIED ALOW COUNT OPTION
1
OUTPUT OPTION FOR STHESS
1
NPF FOR STRESS OUTPUT UPTION
ULTWATE RFSISTANCE INCREMENT (TONS)
WAX RICKS FOR RESISTANCE MICHORY (BPP)
SPECIFIED SEGMENT LENGTH (FT) 100000 000 UNIT MT. (PCF) 42000.00 HEIGHT (LB) HARMER FREICIENCY HARMER ENERGY (FILES) HARMER EXPLOSIVE FIREE (LGS) NUMBER OF HARMER SEGMENTS NUMBER OF MATERIAL TYPES B TABLE 1 -- PROGRAM CONTROL DATA 42.000 (100) TARLE 4 -- PILE SECTION DATA 1000,00 TABLE 3 -- MATERIAL DATA HAMMER DESCRIPTION TARLE 2 -- HAMMER DATA SLACK (IN) MATERIAL TYPE NUMBER SEGMENT 800a

2 G	SECTIONS ADDED FREE GTAVOING PILE(FT)	120,00	٠		
	HATERIAL MALL THICKNESS TYPE (IN)	LENGTH (FT)	TOP	- NUMBER BOTTOM	
- N M G	1,750	100.	0 0 0 0	90 190 290 370	
SOIL					
AUVARA DELITO ARA SINE DAMPENING AR POINT DAMPENING A	P RESISTANCE PERCENTAGES NG RESISTANCE = JOHN THIS PESISTANCE = JOHN TO SING	r.v.			
SOIL BURKE FI	TNIC				
1 2 3 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	10101010101010101010101010101010101010				
•	SPECIFIED 9LOW COUNT DATA				
NUMBER OF SPE	SPECIFIED BLUW COUNTS	3			
BLOWS PER FOOT	TOLERANCE				
	25° 25°				
300	25,				

100 mm # #1 1 03 1 04 1 10 196 71.11. 2115151 . 5 4 4 5 5 4 £2.1217 121451 17.7:: 2-22-5 inger of the property of the p 724275 217661 27727 CHIKEAC PROFIT PELETRATICS - VOLCAS DOD MANNERS 1201 **a** 14 DYIDS, 1255, 412141 BALL TAYCANESSEI, 7514, úSic.S. 114 CC *** -TIP RESISTANCE PERCENTAGE. B. THE SERVICE STREET SO IN STREET Marie Comment of the comment of the 明日 はいのは ひと 日本の The second secon : . ACINE. in t

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0	OTIPE.025, HINTHUM WALL THICKNESSEI, 75IN.		RU = 1.0	
BLE	TABLE 10 SPECIFIED BLOW DATA	H DATA		
-	TIP RFSISTANCE PERCENTAGE	NTAGE-8 14,00		
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had exception editors

SPPING CONSTANT 9200000.00 000 (LB / JN) AANE ERUATION ANALYSIS FOR AZETN, DIAMETER PIRE PILES Me clelland report data for acms 3-pile structure -- boring 1 (SO IN) RESTITUTION VILCAN 060 HAWAFH 42-17, CIAMETER PILES 3-PILE STRUCTURE 250FT PENETHATION == VULCAN 060 HAMMER 0TIP=10 - VINIMUM HALL THICKNESS=1-75IN-490 0 24000cc (TUD) UNIT ATS MODULUS (POST) MAX BUILDS FOR RESISTANCE-BUILD CURVE (RPF) 1.00 300000 000 000 C AREA EFF FOR STAFES CLIPUT CPITON ALTHAR SESTSTANCE INCREMENT (TONS) 60°000877 SPERIFIED SFEWENT LERGTH (FT) AF. 1647 (L.F.) NEW WATERIAL CATA LIBITOR NEW DILL SECTION CATA CRITICA TERRET FYPICSIVE FORCE (LBS)
THYER OF TARES GEORGETS SPECIFIES HATA COTTON SPECIFIES HATA COURT PRING OLTEST (PTIT), FAS STRESS SCHEED OF MATERIAL TYPES IN ** PEGGSA" CONTRUL DATA 75 V 000 NOTITE PARMER DATA LIPTION 1000,000 يَدُنَانِهِ وَنُونَ MATERIAL DATA いし かんない はんじいん エアイを取る ひかのかおびかまうしょ S. AC. TAXMER CATE 78 F. F. B. A. 2264627 *10.836.4 FLY 25,1976 TABLE 1 TAPLE 2 SUGG

ATAN CONTRACTOR SELECTION OF MATE

TOP BOTTOM LENGTH (FT) 2245 120,00 TABLE 6 -- SPECIFIED RLUM COUNT DATA HALL-THICKNESS AUTHER OF SECTIONS CHANGED AUTHER OF SECTIONS ADDED LENGTH OF FREE STANDING PILECETS NUMBER OF SPECIFIED BLUM COUNTS **พูพูพู**พู TULERANCE 35 NUMBER OF TTP RESISTANCE DESIGNANCE SIDE DAMPENING RESISTANCE POINT DAMPENING RESISTANCE SOIL QUAKE FOR SIDE TIP RESISTANCE. SECTION .. MATERIAL NUMBER TYPE . 35,0000-TARLE 5 ... SOIL DATA_ BLOWS PER 150. 200. 250. 300.

TIP RESISTANCE PERCENTAGE = 15,000 TIP RESISTANCE PERCENTAGE = 15,000 TIP RESISTANCE PERCENTAGE = 124 TIP RESISTANCE PERCENTAGE =		OTIPE, 10 , MINIMUM	**	THICKNEGGET	.751W. RU	35					
TIP PRESISTANCE PFECENTAGE = 150.00 ELFV MAX E STRESS TIME N MAX T STRESS TIME N AREA DHAX(N) D(N) VILLE TABLES TIME N AREA DHAX(N) D(N) VILLE TABLES TIME N AREA DHAX(N) D(N) VILLE TABLES TIME N AREA DHAX(N) D(N) VILLE TABLES TIME N AREA DHAX(N) D(N) VILLE TABLES TIME N AREA DHAX(N) D(N) VILLE TABLES TIME N AREA DHAX(N) D(N) VILLE TABLES TIME N AREA DHAX(N) D(N) VILLE TABLES TIME N AREA DHAX(N) D(N) VILLE TABLES TIME N AREA DHAX(N) D(N) VILLE TABLES TIME N AREA DHAX(N) D(N) VILLE TABLES TIME N AREA DHAX(N) D(N) VILLE TABLES TAB	TAB	R MAXIMUM	STRESS	٧.							
ELV MAX C STORS THE N MAX T STRESS TIME N ARFA DHAX(N) D(N) TAS NITE LEASED IN TAS NITE L		RFSIST	F PERCENTAG	-835	00						
Color Colo		DERWANENT GET NUMBER DF BLOW TOTAL INTERVAL	1.F00	2040 - E	INCHES						
0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.0	46. → ►	MAX C STRES	F	AX T	7 I ME	4 08	V J	MAKCHO	O CH)	V(M) 11/36C	
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SPRING CONSTANT 05.0000056 MAVE EQUATION ANALYSIS FOR 42-IN. DIAMETER PIPE PILES MC CLELLAND REPORT DATA FOR ACMR 3-PILE STRUCTURE -- BORING AREA COEF OF ASSESSITUTION 40.00 300 90 VULCAN 060 MAMMER 42-11% DIAMETER PILES 3-PILE STRUCTURE 250-T PENETATION -- VOLCAN 060 HAMMEN 011P=130 "MINIMUM" MALL THICKNESS=1-751N4. 49.0 .0 29200000. **BUJUOUR** (PSI) SPECIFIED ALOW COUNT NOTION

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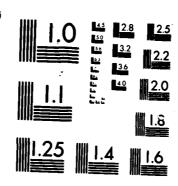
ULITHATE RESISTANCE INCREMENT (TONS)

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SFECIFIED SEGMENT LENGTH. (FIL 50 UNIT MI (PCF) 1000,00 60000,00 PE I GHT NEW MATERIAL DATA MPTION NEW PILE SECTION DATA COTION NEW SOIL DATA SPITCH --- (18) HAMMER EXPLOSIVE FORCE (LBS) TARLE I -- PROGRAM CONTRIL DATA NUMBER OF MATERIAL TYPES 3 NUMBER OF HANNER SFORENTS 42,000. 1007 TABLE 4 -- PILE SECTION DATA NEW HAMMER DATA COPITON. HAMMER ENERGY (FT-LAS). SLACK (IN) TARLE 3 -- MATERIAL DATA MANNER DESCRIPTION TABLE 2 .- HAVMFR DATA HANNER EFFICIENCY - HATERIAL MAY 25,1976 TYPE PILE TYPE SFGMENT PROR •

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FOUNDATION ANALYSIS ERST CORST RIR COMBAT MANEUVERING RANGE OFFSHORE KITT. (U) CREST ENGINEERING INC TULSA OK SEP 76 27-771-97 CHES/NAVFRC-FPO-7612 N62477-76-C-0179 F/G 13/13 AD-A163 522 4/6 UNCLASSIFIED



MICROCOPY RESOLUTION TEST CHART

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SPRING CONSTANT (LM / IN) 9200000.00 WAVE FOUATION ANALYSIS FOR AZ-IN, DIAMETER FIRE FILES MC CLELLAND REPORT DATA FOR ACMA S-FILE STRUCTURE ** BORING MAY 25,1976 AREA COEP OF (80 IN) RESTITUTION 0000 80.0 80.0 96 VULCAN DAD HAMMER AD-14, DIAMFTER PILES S-PILE STRUCTURE 200FT PENFTRATION -+ VULCAN 060 MANNER GTIPE, 025, MINIMUM MALL IMICKNESSEL, TSIN. MODULUS (PBI) 490.00 2900000 SPECIFICO RECHES STRESS INTROL 200 20000000 (PCF) 1000.00 60000.00 1000.00 42000.00 F1047 SPECIFIED BEGMENT LENGTH (PT) NEW PILE SECTION DATA OPTION NEW SOIL DATA OPTION TANKER DESCRIPTION
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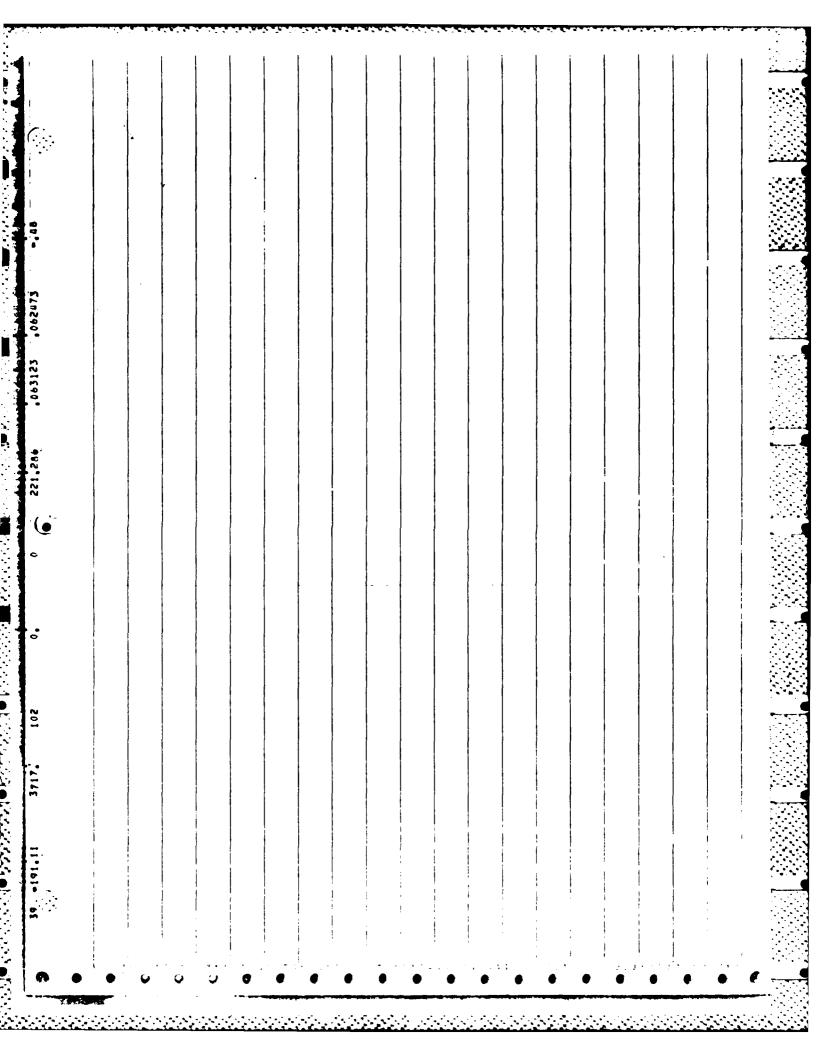
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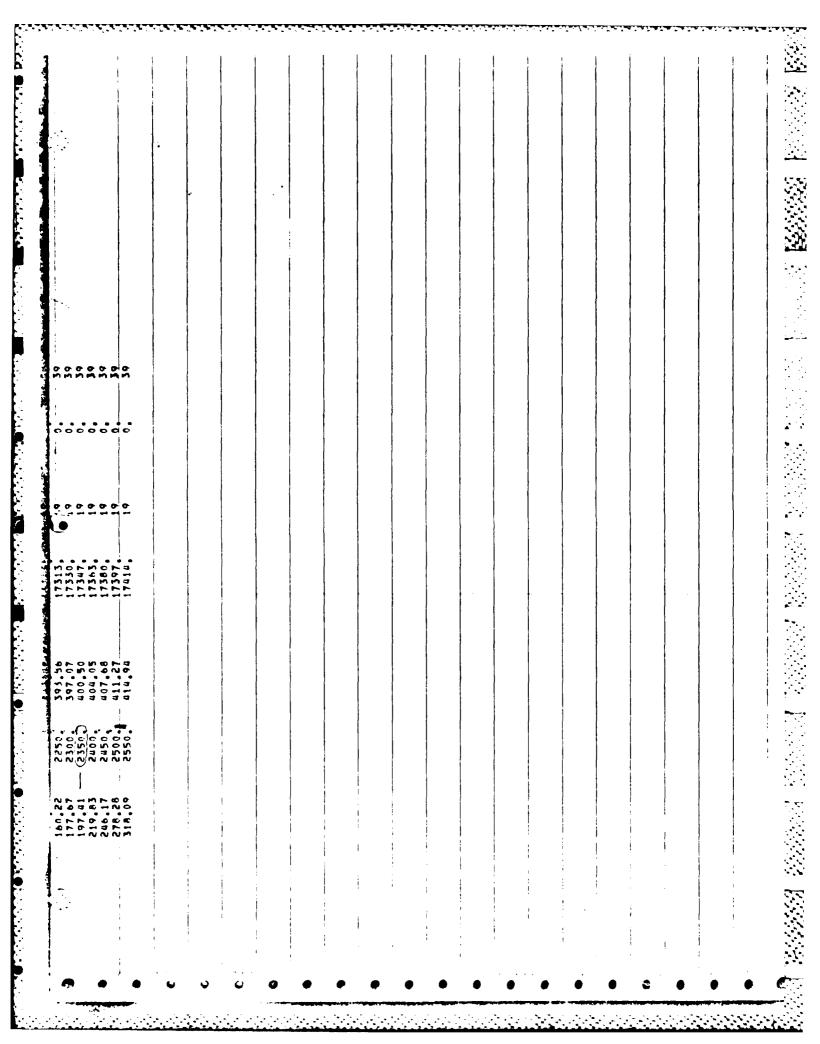
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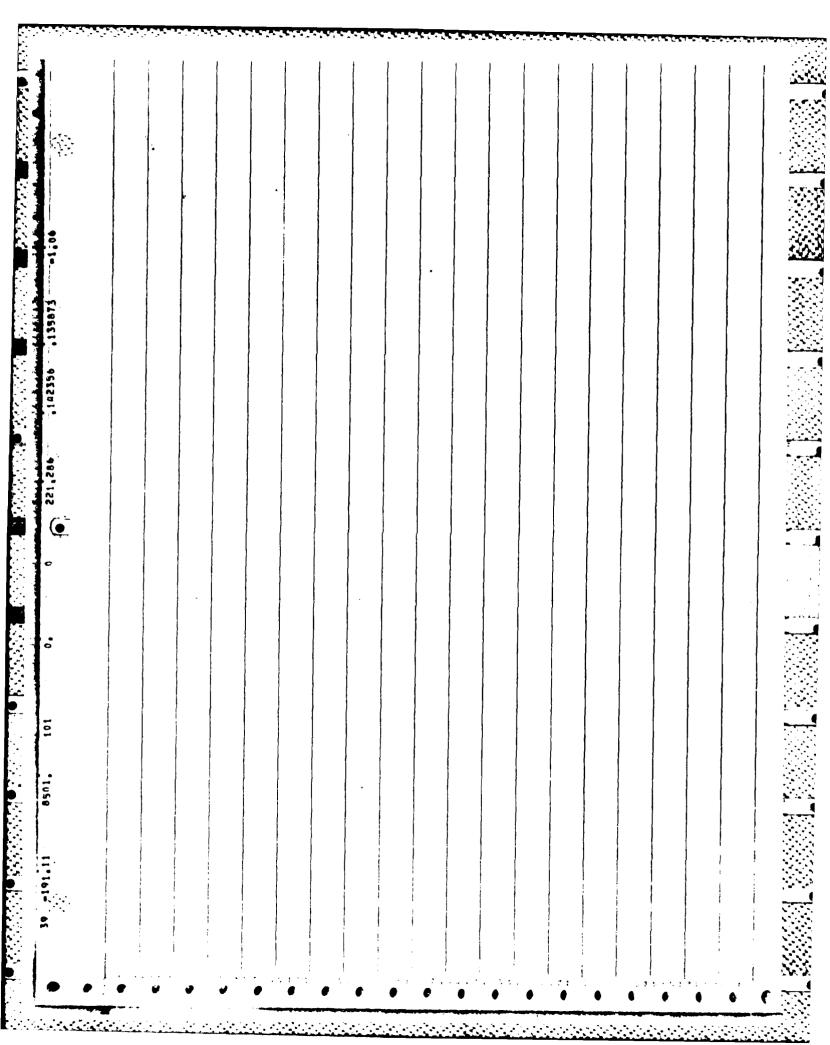


•	9808	42-1N, DIAMETER PILES S-PILE STRUCTURE
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SPRING CONSTANT (LE / IN) 9200000.00 WAVE EQUATION AVALYSIS FOR 42-IN. DIAMETER DIPE PILES MC CLELLAND REPORT DATA FOR ACHR 3-PILE STRUCTURE -- BORING AREA COEF OF ISO IN) RESTITUTION 50.0 30 VULCAN DED HAMMER 42-IN, DIAMFTER PILES 3-PILE STRUCTURE 200FT PENETHATION -- VULCAN 060 HAMMER GIIPE,30 MINIMUM.MALL THICKNESSEI,TSIM, 490.0 29000000 MODUL US (PS1) MAX BLOWS FOR RESISTANCE BLOW CURVE (BPF) SPECIFIED SEGMENT LENGTH (ET) 000 •0•00 300000 ULTIMATE RESISTANCE INCREMENT (TONS) UNIT HT. 1000.00 60000.00 WEIGHT (LB) NEW PILE SECTION DATA OPTION NEW SOIL DATA OPTION HAMMER EFFICIENCY HAMMER ENERGY (FIGELES) HAMMER EXPLUSIVE FORCE (LBS) Number of Hammer Segments BPF FOR STRESS DUTPUT OPITON SPECIFIED ALOW COUNT OPTION OUTPUT OPTION FOR STRESS TABLE 1 .- PROGRAM CONTROL DATA NUMBER OF MATERIAL TYPES A 42,000 NEW HAMMER DATA OPTION NEW MATERIAL DATA OPTION (100) TABLE 4 -- PILE SECTION DATA TABLE 3. . . HATERIAL DATA HAMMER DESCRIPTION TABLE 2 .. HAMMER DATA SLACK 315 HATERIAL SEGMENT NUMBER KAY 25,1976 PROB

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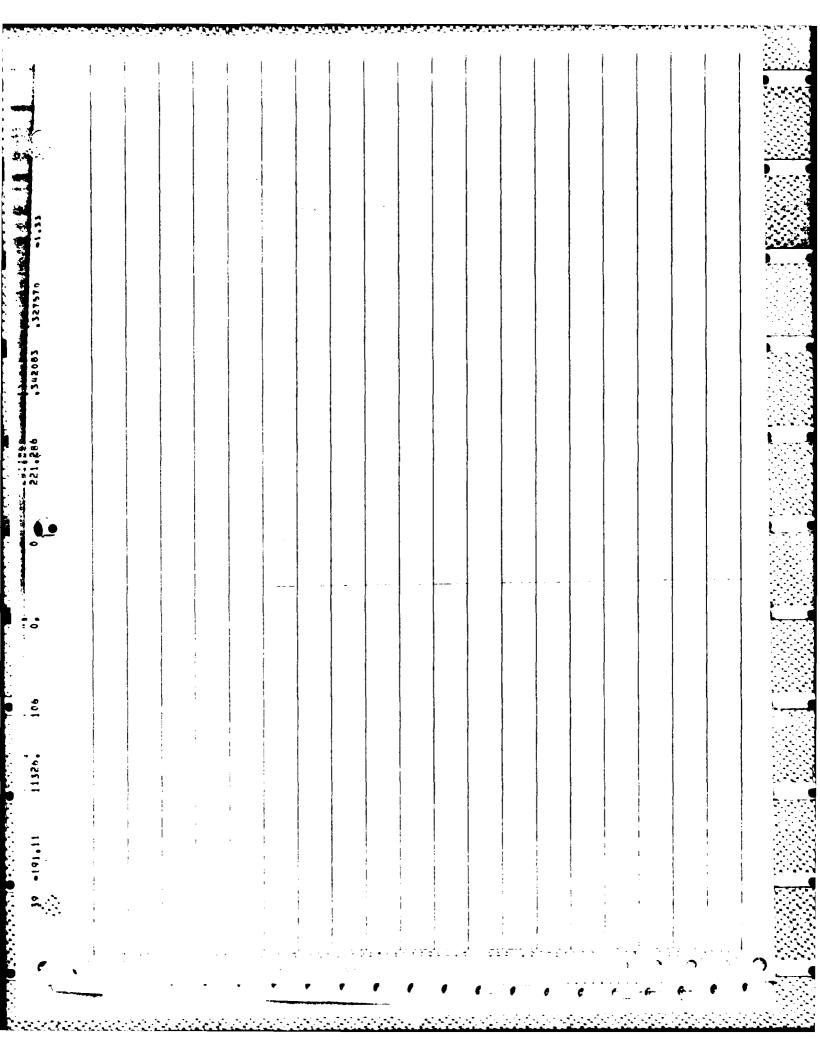


TABLE 9 ** RESISTANCE* TO PESISTANCE PE					
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PROB 2007 PERETRATION VOICEN GO MANAGEES 3-9116 STRUCTURE OTTP=,30 -41M19WN WALL PRICENESSHI,751W, RU = 50 TABLE 10 SPECIFED SLOW OATA TABLE 10 SP					(\$
200FT PENETRATION == "VOCAN DOS HAWER PILES SAPILE STRUCTURE OTID=,30,-1NINUM MALL THICKNESSB1.751N, RU = 50 TABLE 10 == SPECIFIED BLDW DATA TID=RESISTANCE PERCENTAGE = 50,00 RUD=SPER RESISTANCE RESISTANCE PERCENTAGE = 50,00 RUD=SPER RESISTANCE 150,41 150,41 150,41 150,41 150,41 150,51 245,75 1650,51 285,15 1650,51	!				
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Pile Driving Resistance Curves

Pile Diameter - 42 in.

Minimum Wall Thickness - 1.00 in.

Penetration - 150 ft.

- 200 ft.

Hammer - Vulcan 560

Quake Factor, tip - .025 in.

- 0.10 in.

- 0.30 in.

SUNITED COMPUTINGS 57, APEX/SL 8,2,0 9599999999 96/08/76. 00000000 16,04.07.

MAVE EQUATION ANALYSIS FOR 42-IN. OIAMETER PIPE FILES
HC CLELLAND SOIL REPORT DATA FOR ACMR 3-PILE STRUCTURES -- BORING SITES SA +
B JUNE 1976 SPRING CONSTANT 00.000024 42-14, DIAMETER PILES MLMA10SFT 3-PILE STRUCTURES 150FT PENETRATION 4- VULCAN 560 HAMMER 021PH,005,MINIMUM WALL THICKNESSE1,00 IN. RU # 1 300.0 AREA COEF OF (SO IN) RESTITUTION 96 VULCAN SEG HAMMER HODULUS (PSI) 490,0 29000000 CLTIMATE RESISTANCE INCHEMENT (TONS)
MAX BLOWS FOR RESISTANCE-MICH CURVE (BPF)
SPFCIFIED SEGMENT LENGTH (FT) 1.00 00.0-300000,00 UNIT MT. (PCF) 42000,00 HEIGHT (LB) NEW PILE BECTION DATA OPTION RPF FOR STRESS OUTPUT CIPTION NUMBER EXPLOSIVE FORCE (LBS) NOILED WITH COUNT OFFICE NUMBER OF MATERIAL TYPES & TABLE 1 -- PROGRAM CONTROL DATA 42,000 NEW MAVWER DATA OPTION NEW MATERIAL DATA NPTION CUITAUT OPTION FOR STRESS (100) TABLE 4 -- PILE SECTION DATA HAMPER ENERGY (FT-LAS) 1000.00 TABLE 3 .- MATERIAL DATA TABLE 2 .. HAMMER DATA HAMMER DESCRIPTION SLACK HAMMER EFFICIENCY HATERIAL SECTEMP NUMBER **BDBa**

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FOR STANDING PROPERTY GREATES SECTIONS

1 JOHN PERFFAITUR -- VOLCAN SAO HAMMER 9.0

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MANY FOURTION ANALYSIS FOR AZEIN, DIAMETER PIPE FILES
FO CLFLLAND SOIL REPORT DATA FOR ACMR SAPILE STRUCTURES ** BORING SITES SA
B JUNE 1976 SPAING CONSTANT 00.000020 ACAIN, DIAMFTER PILES MLMHIOSFT SAPILE STRUCTURES 150FT PERETRATION SA VULCAN SAO MAMMER DITTER 1100, MINIMUM FALL THICKNESSH, SO IN. RU & 3 (ST IN) RESTITUTION VULCAN 366 HANNE 490,0 29000000. MODULUS (PSI) VAX BLUBS BUR SPOTOTANCE-BLUB CURVE (BBF) SPECIFIED BEGLENT LENGTH (FT) 300000.00 LON FOR STRESS CUTPUT OPTION (TONG) CADA 42000,00 TALLER ENFOGY (FT-LAG)
TALLER EXPLOSIVE FORCE (LBS)...
TOLLER OF HALMER SEGMENTS PEIGHT NEW MATERIAL DATA OPTION
NEW PILE SECTION DATA COTION
NEW SOIL DATA OPTION
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TABLE & SE PILE SECTION DATA

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MAVE EQUATION ANALYSIS FOR 42-IN. DIAMETER PIPE PILES

MC CLELLAND SOIL REPUNT DATA FOR ACMR 3-PILE STRUCTURES -- BORING SITES 3A +

A JUNE 1978 SPRING CONSTANT 6200000,00 RU ■ 50 D2-1N, DIAMETER PILES MLWAIDSFT 3-PILE STRUCTURES 150FT PENETRAIION -- VULCAN 560 MANNER GTIPE,500,MINIMUM WALL THICKNESSBI,00 IN. RU E SI •0.00 AREA COEF OF (90 14) RESTITUTION 275. 50.0 800. 6 9 VIILEAN 560 HAMMER 400.0 2000000. **BUJUOOM** (18d) 300000° 00°00° CACH BLUE SON SEGMENTANDERS CUSAN SINE XEE 1.00 CENTRATE DESIGNANCE INCREMENT (40NB) UNIT MT. (PCF) 42000.00 00.00000 NE 16HT (L8) SPECIFIED SFOMFNT LENGTH (FT) NEW MATERIAL DATA OPTION NEW PILE SECTION DATA OPTION NEW SHIL DATA OPTION SPECIFIED HEDM COULT OPTION OUTPUT COTTON FOR STRESS FOR STRESS .. PROGRAM CONTROL DATA 42,000 OF HAMMER SECRETA NUMBER OF MATERIAL TYPES HALLER DESCRIPTION
HALLER EFFICIENCY
HALLER ELERGY (FTELPS)
HALLER EXPLOSIVE FORCE (401) TABLE & -- PILE SECTION DATA NEW MAMPER DATA OPTION 1000,00 TARLE S -- MATERIAL DATA SLACK (IN) SE NAMPER DATA MATFRIAL TYPE SFGVFAT A BREOK TABLE 1 TABLE 2 PRINE

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TABL	TABLE 10 SPECIFIED BLOW DATA	DATA		
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٠ MAVE EQUATION BRALYSIS FOR 4221N, DIBNETER PIPE PILES
MC CLELLAND SOIL REPORT DATA FOR ACMR SAFILE STRUCTURES .. BORING SITES SA B JUNE 1976 SPRING CONSTANT (LB / IN) 05.00005.00 RU # 14 BRAIN, DIAMETER BILEM HEABIOSFT BEPILE BIRUCTURES ROOFT PENETRATION 44 VULCAN 560 MANMER GIIPR, 025, MINIMUM MALL THICKNESSEI, 00 IN. RU R 1/ AREA COEF OF (SO IN) RESTITUTION 500.00 90 VOLCAN S60 HAMMER MODULUS (PSI) 490.0 29000000 ULTITATE REBISTANCE INCREMENT (TONG)
TAX BLOTO FOR RESISTANCE-BLOM CURVE (BPF)
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TABLE 4 .- PILE SECTION DATA

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TABLE B HAXIMUM STRESS DATA TIP RESISTANCE PERCENTAGE B 14,00 IN, RU B 16 PERMANENT SET OF PILE B 041,00 IN, RU B 16 O 00 0 001676	TABLE 8 MAXINUM WALL TABLE 8 MAXINUM STRESS DA TIP RESIDANCE PERCENTA TIP RESIDANCE PERCENTA NUMBER OF SCOTO STRESS TIME N TO 00 0 00 00 00 00 00 00 00 00 00 00 00	THICKNESS#1.00 IN. RU B 16	# 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	X X X X X X X X X X X X X X X X X X X	10. M. M. M. M. M. M. M. M. M. M. M. M. M.	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	
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TABLE 1 PROGRAM CONTROL DATA	
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ULTIMATE RESISTANCE INCREMENT (TONS) MAX BLOWS FOR RESISTANCE—BLOW CURVE (BPF) 500. SPECIFIED BEGMENT LENGTH (FT) -0.00	
HAMMER DESCRIPTION VOLCAN 560 HAMMER LAHKER ENTITIENCY TALKER ENTITIENCY TALKER ENTITIENCY TALKER ENTITIENCY TALKER ENTITIENCY TALKER EXPLUSIVE FORCE (LBS) NUMBER OF HAMER	
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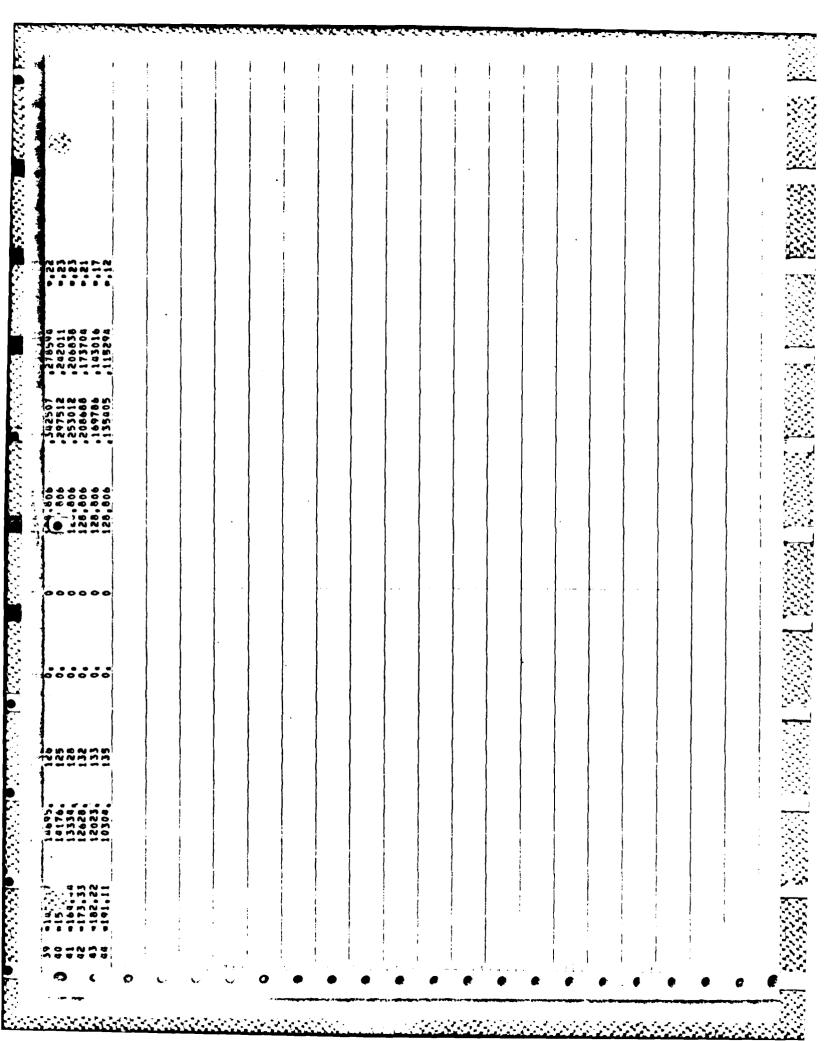
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S ** SOIL DATA S ** SOIL DATA SUUMBER OF TIP RESISTANCE * JSIDE SIDE DAMPENING RESISTANCE * JSIDE SOIL GUAKE FOR STOE * GSIDE SOIL GUAKE FOR POINT * OPOINT * 10 TIP RESISTANCE SS.0000 TABLE 6 ** SPECIFIED BLOW COUNT DATA WHEER OF SPECIFIED BLOW COUNTS & 25. SOO.		-	00041	•00	200	200			
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٠ MAVE EQUATION ANALYBIG FOR AZTIN, DIANETER PIPE PILES
ME CLELLAND SOIL REPORT DATA FOR ACHR 3-PILE BIRUCTURES -- BORING SITES SA
B JUNE 1976 SPRING CONSTANT (LB / IN) 00,0000054 AU = 50 A2-1N, DIAMETER PILES MEMBIOSFT SAPILE STRUCTURES 200FT PENETRATION -- VULCAN 560 MAMMER GITAS, 500, MINIMUM MALL THICKNESSEI, 00 IN. RUE S AMEA COEF OF (80 IN) RESTITUTION 96 VULCAN SEG HAMMER MODUL UB 490,0 29000000 NEW BOIL DATA OPTION
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TATHER EVERGY (FT-LGS)
TATHER EXPLOSIVE FORCE (LGS)
VLINGER OF TATHER SECRETS NUMBER OF MATERIAL TYPES & TABLE 1 -- PROGRAM CONTROL DATA 42,000 (100) TABLE & -- PILE BECTION DATA 1000,00 TABLE 3 -- MATERIAL DATA SLACK (IN) TABLE 2 .. HAMMER DETA-HATERIAL TYPE SECHENI NUMBER PR06

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	PROB 2005T PEVETRATION VULCAN 560 HAMMER	DINDH, MOO, MININCH WALL THICKNESSHI, OO IN.	TABLE 10 ** SPECIFIED BLOW DATA TIP RESISTANCE PERCENTAGE #	BLOWS PER	1		

Pile Driving Resistance Curves

 Pile Diameter
 - 42 in.

 Minimum Wall Thickness
 - 1.25 in.

 Penetration
 - 150 ft.

 - 200 ft.

 Hammer
 - Vulcan 560

 Quake Factor, tip
 - .025 in.

 - .10 in.

 - .30 in.

OR 42-IN, DIAMETER PIPE PILES DATA FOR ACMR 3-PILE STRUCTURES BORING SITES SA + 4	S MLWRIOSFT 3#PILE STRUCTURES VULCAN 560 HAMMER IL THICKNESSH1.25 IN. RU # 14	1. I	10N 1 1 10N 1 1 10N 1 1 10N 1 1 10N 1 1 10N 1 1 10N 1 1 10N 1 1 10N 1 1 1 1	TON 275	VULCAN S60 HAMMER	AREA COEF OF SPRING	(80 IN)	10	UNIT WT. MODULUS (PCF) (#S1) 490.0 29000000.	A SMIT
MAVE EGUATION ANALYSIS FOR 4 HC CLELLAND SUIL REPURT DATA 8 JUNE 1976	AZ-IN. DIAMETER 150FT PENETRATIO GTIPE 025, MINIMU	1 PROGRAM CONTROL DATA PILE TYPE NEW HAMMER DATA OPTION	H MATERIAL DATA OPTIU H PILE SECTION DATA O H SOIL DATA OPTION ECIFIED 9LOW COUNT OP	DUTPUT OPTION FOR STRESS BPF FOR STRESS OUTPUT OPTION ULTHATE RESISTANCE INCREMENT (MAX BLONS FOR RESISTANCE-BLOW C 'SPECIFIFO SEGMENT LENGTH (FT)	TABLE 2 00 HAMBER DATA MANTER DESCRIPTION NAMER EFFICIENCY NAMER FFFICIENC	EXPLOSIVE FORCE OF HAMER SEGMEN	1000,000	3 MATERIAL DATA NUMBER OF MATERIAL TYPES	MATERIAL (TOD) TYPE 1 42,000	A PILE SECTION DATA

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RU # 35

TARLE 1 -- PROGRAM CONTROL DATA

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TABLE & -- PILE SECTION DATA

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S ** SOIL DATA VUMBER OF TIP RESISTANCE PERCENTAGES 1 SIDE DAMPKING RESISTANCE - JSIDE .1 SOIL DUAKE FOR SIDE - GRIDE .1 SOIL QUAKE FOR BOINT - GRIDE .1 SOURCE TO SPECIFIED BLUM COUNT DATA FOOT TOLERANCE .25. 250. 250. 250. 250. 250. 250. 250.			11.000	80.	180	310	
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#55.00 16970 92 0. 0 190.852 62250 307973 #1. #55.55 16915 94 0. 0 190.852 585802 307973 #1. #55.55 16915 97 0. 0 190.852 585802 307853 #1. #70.00 18356 99 0. 0 160.025 456705 275321 #1. #70.80 17459 102 0. 0 160.025 456705 275321 #1. #70.80 17459 102 0. 0 160.025 456705 250708 #1. #70.55 15187 108 0. 0 160.025 305156 205708 #1. #70.55 15187 108 0. 0 160.025 305156 205708 #1. #72.52 11945 115 0. 0 160.025 167491 12358 #1.		36.6	17440	► 6	ē o	9 C N	e. ≪ R. K.	2 7	1	<u>.</u> .	
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#70.00 8356. 99 0 100.025 547750 500465 61.00.000 18356. 99 0 100.025 556281 291661 61.00.000 185675 527521 61.00.000 185675 185675 185675 185675 185675 185675 1867. 108 0 1867.000 1867.0		53,3	513	7 7		٥ (95	2	0555	-	
#76.89 17459, 102 0, 100,025 255751 #1, 105.025 106.025 106.025 255751 #1, 106.025 106		10.0	400	00			6	2	900	. .	
#87.76 16636 106 0. 0 160.029 400.34 25.0331 *** *******************************		73.8	459	0		> 0	20	22	7166 7532		
#96.67 160Rd, 108 0, 0 160,025 352993 230089 6, 6105,025 15187, 108 0, 0 160,025 303156 203788 6, 110,44 14095, 110 0, 0 160,025 25502 176488 6, 123,53 129Re, 119 0, 0 160,025 258180 149238 6, 132,22 11948, 115 0, 0 160,029 167491 12338 6,	81	87.7	636	0		0	9	2	5433	: ;	
#103,525 13037 108 0, 0 160,025 303156 203788 8, 110,025 11095, 176288 8, 17	. z	900	16084	9		0	0.5	2:	3008	24.	
#123,33 12986, 115 0 0 0 160,025 209180 149238 F.	e in	1000	12157	o		0	50	2 2	0378		
#522622 11943, 115 0. 0 160,025 119491 123308 m		23,3	986	· ·		0	20.	9	1923		
		132,22	943	~ -		• •	20	0 3	2330	•	

# RESISTANCE #BLDA CURVE T. PESISTANCE DYNAMIC PT TOTAL #TONS FORCE#TONS 88 50 67 31 61 100 129 50 55 150 186 71 64 250 287 33 02 350 400 410 58	DATA 35.00	C		
1. PESISTANCE DYNAMIC TOTAL STONS FORCENTAGE 88 50 120 55 55 55 50 239 1	35.00			
1. PESISTANCE DYNAMIC TOTAL STORMS FORCEST 56 100 129 5 55 150 239 1 59 250 287 3 59 350 410 5				
61 100 1100 67 100 100 100 100 100 100 100 100 100 10	MAX C STRESS LBS/SQ.IN. NO.	SEG MAX T	STRESS	ØEG
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29 350 372 7 69 400 410.5	18152		700	200
.29 350, 372,7 .69 400, 410,5	18210		726.	30
	18236			0 6
.26 450, 445,4	19278		222	30
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WAVE EDUATION ANALYSIS FOR AZAIN, DIAMETER PIPE PILES
HC CLELLAND SOIL REPURT DATA FOR ACMS 34PILE STRUCTURES 4* BORING SITES 3A JUNE 1976

→ システンションは、1997年間の1997年の1997年間では、1997年の1

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TABLE 1 ** PROGRAM CONTROL DATA	
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TABLE 2 HAMRR DATA	
MAMMER DESCRIPTION VULCAN S60 HAMMER MAMMER EFFICIENCY (FT-LAS) 300000,00	
EXPLISIVE FORCE (LBS) OF MAMMER SEGMENTS	
SEGMENT SLACK WEIGHT AREA COEFOF SPRING CONSTANT NUMBER (IN) A (LH) (SG IN) RESTITUTION (LB / IN)	TANT
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TABLE 3 MATERIAL DATA	
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P408 150FT PENETRATION ***	42-IN. DISMETER PILES MUMBIOSET 3-PILE BIRUCTURES VULCAN 560 MAMMER	
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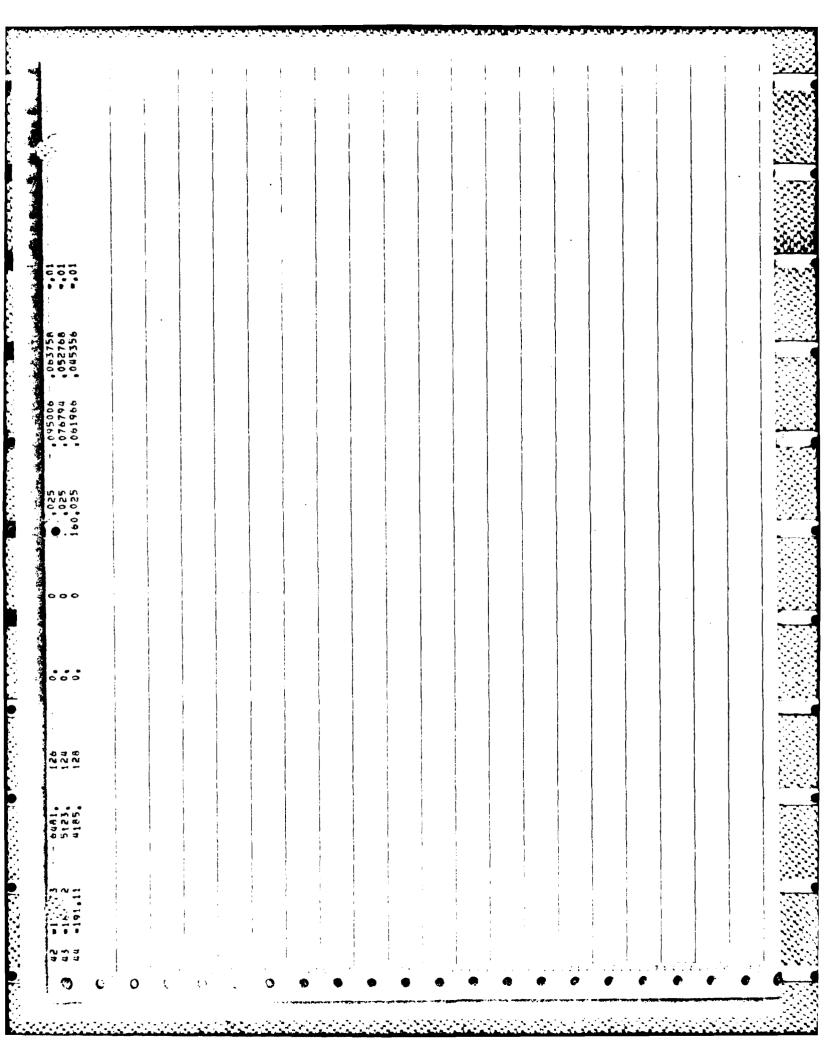
WAVE EQUATION ANALYSIS FOR 42-IN. DIAMETER PIPE PILES
WE CLELLAND SUIL REPUBLICATA FOR ACMR 3-PILE STRUCTUPES -- BORING SITES 3A +
B. JUNE 1976 SPRING CONSTANT (LB / IN) 00,000029 RU # 14 ACSTIN, DIAMETER PILES MENHIOSET SEPILE GIRUCTURES ZOOFT PENETRATION ** VULCAN 560 MANNER CITPA,025, MINIMUM WALL THICKNESSKI,25 IN, RU # 14 AREA COEF OF (SQ IN) RESTITUTION 275 90 VULCAN SEO HAMMER 440.0 2400000 MUDDILUS \$00000° 00 ULTIMATE REGISTANCE INCREMENT (TONS)
MAX BLUMS FOR RESISTANCE-BLOW CURVE (BPF)
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TARKER ENERGY (FTELSO)
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TOTAL OF TARKER SPECIFIED ALOW COUNT OPTION NUMBER OF MATERIAL TYPES & -- PROGRAM CONTROL DATA 42,000 CUIPUT CPTION FOR STRESS (100) TABLE 4 -- PILE SECTION DATA 1000.00 TABLE 3 .. MATERIAL DATA HAWER OF SCRIPTION 3L ACK TABLE 2 -- HAMMER DATA (12) MATERIAL TYPE いといといいの RC*BER BURG 3

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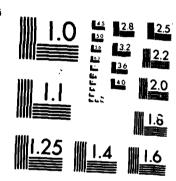
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200FT PENETRATION -- VULCAN 560 HAMMER OTIDE 025,4IVINUM WALL IMICKNESSEL 25 IN. TIP RESISTANCE PERCENTAGE B 14 00 1750 1950 1950 REGISTANCE TONS TABLE 10 ** SPECIFIED BLOW DATA PLOWS PER 127.60 200.35 235.54 J ** BORING SITES 3A SPRING CONSTANT 00.0000029 N. 42*1% DIAMETER DILES "MUMETOSET JAPILE STRUCTURES"
200FT PENETAATION ** JULOAN 560 HAMMER
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FOUNDATION ANALYSIS ERST COAST AIR COMBAT MANEUVERING RANGE OFFSHORE KITT..(U) CREST ENGINEERING INC TULSA OK SEP 76 27-771-97 CHES/NAVFAC-FPO-7612 N62477-76-C-0179 F/G 13/13 AD-A163 522 5/6 UNCLASSIFIED NL



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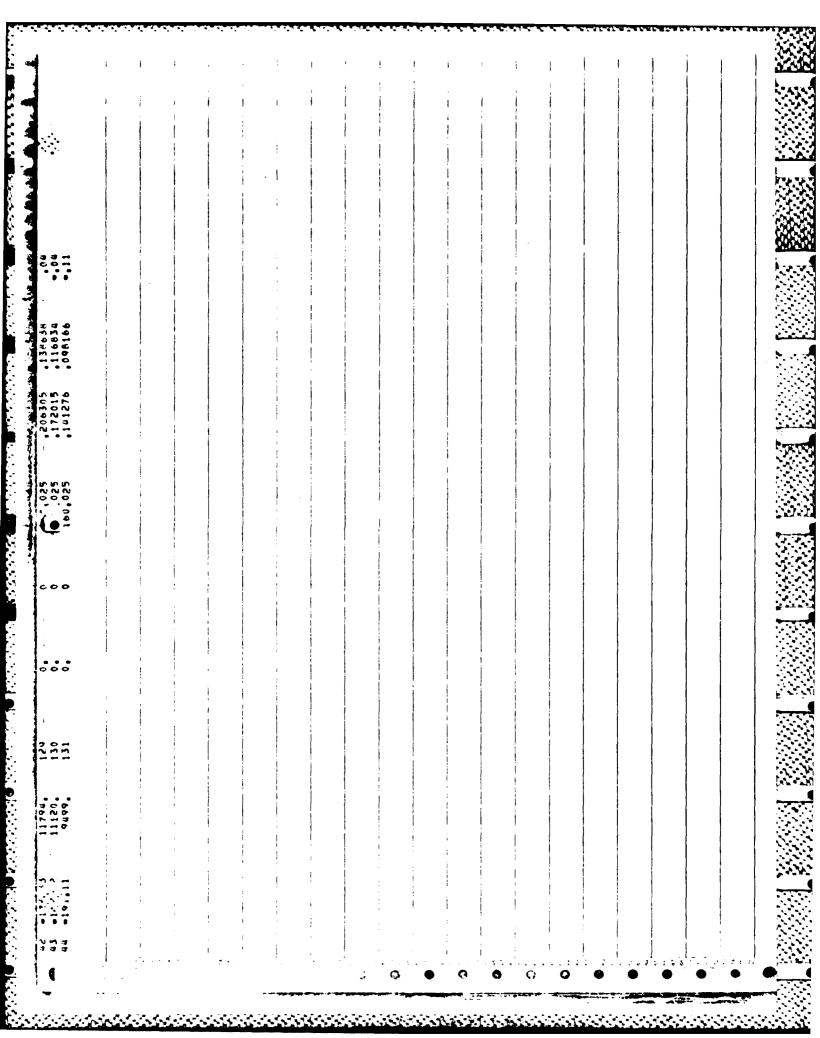
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DIIPB,100,MINIMUM WALL THICKNES TABLE 10 SPECIFIED BLOW DATA	SPECIAL SERVICE TEACHER.	130.17 218.34 245.67 290.72			

+ WAVE EQUATION ANALYSIS FOR 42-IN, DIAMETER PIPE PILES
MC CLELLAND SOIL REPORT DATA FOR ACHR 3-PILE STRUCTURES -- BORING SITES SA
A JUNE 1976

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SPRING CONSTANT 6200000g00 42-1N, DIAMETER PILES MEMBIOSET 3-PILE STRUCTURES
200FT PENETRATIUN -- VULCAN 560 HAMMER
011P2,300,MINIMUM WALL THICKNESSE1,25 IN, RU = 50 CSO IN) RESTITUTION 000 VULCAN SON HAMMER MODUL US 440.0 24000000 ULTIMATE RESISTANCE INCHEMENT (TONS)
MAX ALIMS FOR RESISTANCE-BLOW CURVE (BPF)
SPECIFIED SEGMENT LFNOTH (FT) 000 UNIT MT. (PCF) 1000,00 60000,00 HALLER DESCRIPTION
HALLER EFFICIENCY
HAMLER ENERGY (FT-193)
HALLER EXPLOSIVE FURCE (183)
NUMBER OF HAMMER SEGMENTS ME 16HT (LB) SPECIFIED ALOW COUNT OPTION TOTALS OF THESS TOTAL NUMBER OF PILE SECTIONS NUMBER OF SECTIONS CHANGED HUMBER OF SECTIONS CHANGED NEW WATERIAL DATA OPTION
NEW PILE SECTION DATA OPTION
NEW SOIL DATA OPTION APF FOR STRESS JUTPUT DETTION NUMBER OF MATERIAL TYPES A TABLE 1 .- PRIJGRAM CONTROL DATA 42,000 TABLE 4 -- PILE SECTION DATA (100) PILE TYPE NEW HAWHER DATA OPTION TABLE S .. MATERIAL DATA SLACK (IN) TABLE 2 -- HAMMER DATA MATERIAL SEGMENT 90'dd ٠ O

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N MATERIAL RATERIAL			SOIL DATA	NUTBER OF TIP REGISTANCE PER GIOGE DAMPENING REGISTANCE A COLLAY DAMPENING REGISTANCE A	QUAKE FOR STD	TIP RESISTANCE PERCENTAGE	20000	•	ER OF SPECIFIED	S PER	150.	300					
ACETADA.	E N 0-		TABLE 5	BOTO PARTOR	SOIL			TABLE	NUMBER	BLOWS F			9	•			

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		9	274.91	21.2	0	4173970	
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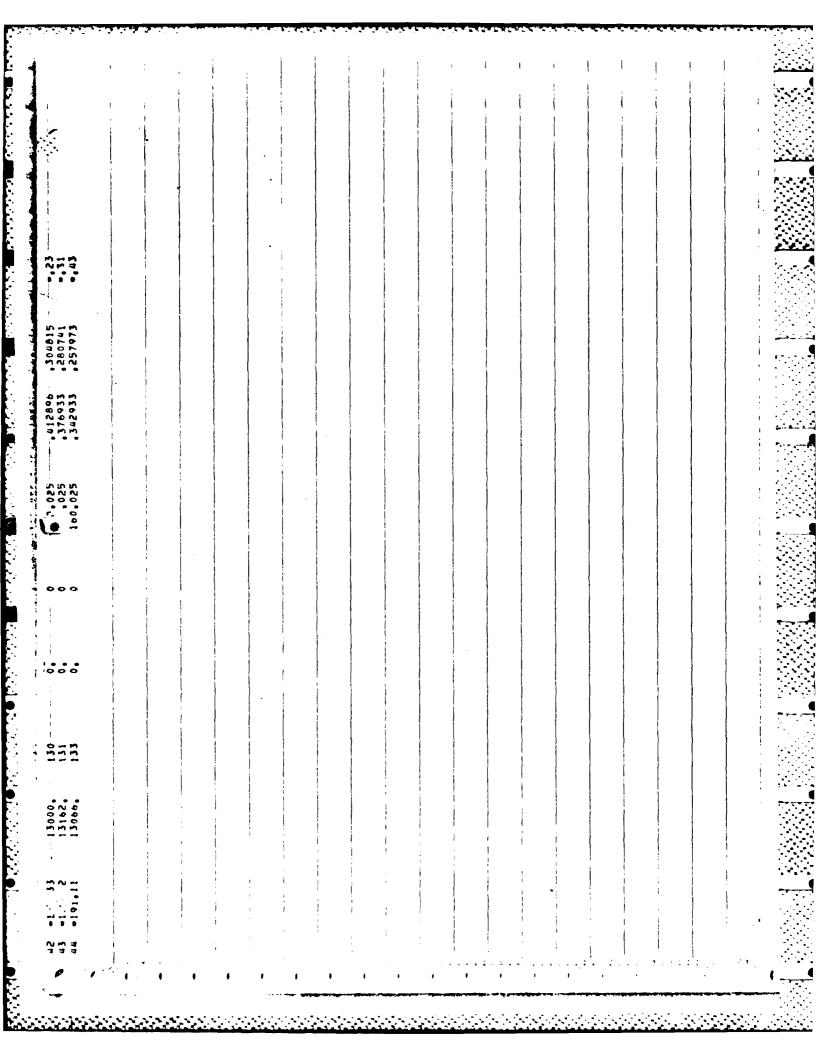
STRUCTURE OF COLUMN ALEBROSPY WINDS AT SUCCESSION PROFESSION OF COLUMN SOCION SOCIETATION SOCIONS SOCIONI SOCIONI PACE

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TABLE B SE MANIMUM STRESS DATA

TIP RESISTANCE PERCENTAGE . SO.00

PERMAKENT SET UP FILE & GOARD INCHES NUMBER OF HOMES BEN FORT & REGINE TOTAL INTERVALS



200FT PENETRATION -- VULCAN 560 HAMMER SEG 3 3 3 LAS C STRESS SEG MAX T STRESS LAS NO. LAS SO. TN. NO. 2655 2052 10062 10102 20103 14985, 9811 8349 7027 5810 1697 30 . DIIPB SOO, MINIMUM WALL THICKNEGGBI 25 IN. 18394 18409 18423 18438 17291 17852 18295 18321 18525 18536 18547 8477 8502 8513 8556 18575 18575 18575 18504 18520 18452 18361 50.00 TABLE 9 -- RESISTANCE-BLOW CURVE DATA BLOWS/FT, RESISTANCE DYNAMIC PT TOTAL-TONS FORCE-TONS TIP RESISTANCE PERCENTAGE 78.50 78.50 92.19 115.59 125.50 125.01 279.51 58.65 43.72 49.83 57,31 PROB

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GTIPE, 300, HINIHU	GTIPE, 300, MINIMUM WALL THICKNESSEL, 25 IN.	25 IN. RU # 50		
TABLE 10 SPECIFIED BLUW DATA	D BLUM DATA			
TIP REGISTANCE PERCENTAGE	PERCENTAGE # 50.00	00		
BLOAS PER FOOT	REGISTANCE			
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243,05				
			•	

Pile Driving Resistance Curves

Pile Diameter	- 42 in.
Minimum Wall Thickness	- 1.50 in.
Penetration	- 150 ft.
	- 200 ft.
Hammer	- Vulcan 560
Quake Factor, tip	025 in.
	10 in.
	30 in.

	B,2,0	•	**************************************	RR 88 88 LL RR 88 88 LL RR 88 88 LL	48 6668488686868 866688888	25 25 25 25 25 25 25 25 25 25 25 25 25 2	AR RES BES BES LI LILILILILI BES BES BES BES BES BES BES BES BES BES	
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.. BURING SITES 3A SPRING CONSTANT (LB / IN) 00.0000054 7. HIMETOSFT 3-PILE STRUCTURES 275. 50.0 300. AREA COEF OF (SO IN) RESTITUTION 2 WAVE ... ATTOW ANALYSIS FOR 42-IN. DIAMETER PIPE PILES MG CLELLAND SOIL REPORT DATA FOR ACMR 5-PILE STRUCTURES 99. VULCAN S60 HAMMER 490.0 29000000 MODULUS (ISd) MAX HLOWS FOR RESISTANCE -BLOW CURVE (BPF) .00 300000,00 UCTIMATE RESISTANCE INCHEMENT (TONS) UNIT #T. NUMBER OF SECTIONS CHANGED NUMBER OF SECTIONS ADDED LENGTH OF FREE STANDING PILE(FT) 42000,00 WEIGHT (LB) TOTAL NUMBER OF PILE SECTIONS SPECIFIFD SEGMENT LENGTH (FT) NEW PILE SECTION DATA UPTION HAMMER EFFICIENCY MAMMER ENERGY (FTGLHS) HAMMER EXPLOSIVE FORCE (LHS) SPECIFIED ALUM COUNT OPTION NUMBER OF MATERIAL TYPES . -- PROGRAM CONTROL DATA NUMBER OF HAMPER SEGMENTS 42,000 DUTPUT OPTION FOR STRESS TABLE 4 -- PILE SECTION DATA (100) 1000.00 .. MATERIAL DATA SLACK (IN) TABLE 2 SE HAMMER DATA MATERIAL SEGMENT NUMBER TABLE 1 TABLE 3 PROB

FOR ENERGY DATA WENT ELEV SLACK MEIGHT AREA COEF RSTITU SPR STI 1 0 0 0 0 1000 0 0 0 0 0 0 0 0 0 0 0 0	ENT EEEV SLACK METGHT AREA COEF RSTITU SPR 160.00 1000.00 0.00 1.00 0.00 1.00 0.00 1.00 0.00 1.00 0.00 1.00 0.00 1.00 0.		# FG # E N E SEGUENT DATA	A B B B B B B B B B B B B B B B B B B B	# + + + + + + + + + + + + + + + + + + +	SPR STIFF	
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WAVE FOUATION ANALYSIS FOR 42-IN. DIAMETER PIPE PILES HC CLELLAND SOIL REPURT DATA FOR 3-PILE STRUCTURES ... BORING SITES 3A + R JUNE 1976

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TABLE 4 .- PILE SECTION DATA

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WAVE EQUATION ANALYSIS FOR 42-IN, OIAMETER PIPE PILES MC CLELLAND SOIL REPORT DATA FOR ACMR 34-PILE STRUCTURES +* BORING SITES 3A B JUNE 1976

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PRIMA

42-IN. DIAVETER PILES RESIDENT SEPILE STRUCTURES ANDET PERFETABILM -- VULCAN 560 HAMPER DITOR 100 MINIMUL MALL INICKNESSR1,50 IN. RU E 35

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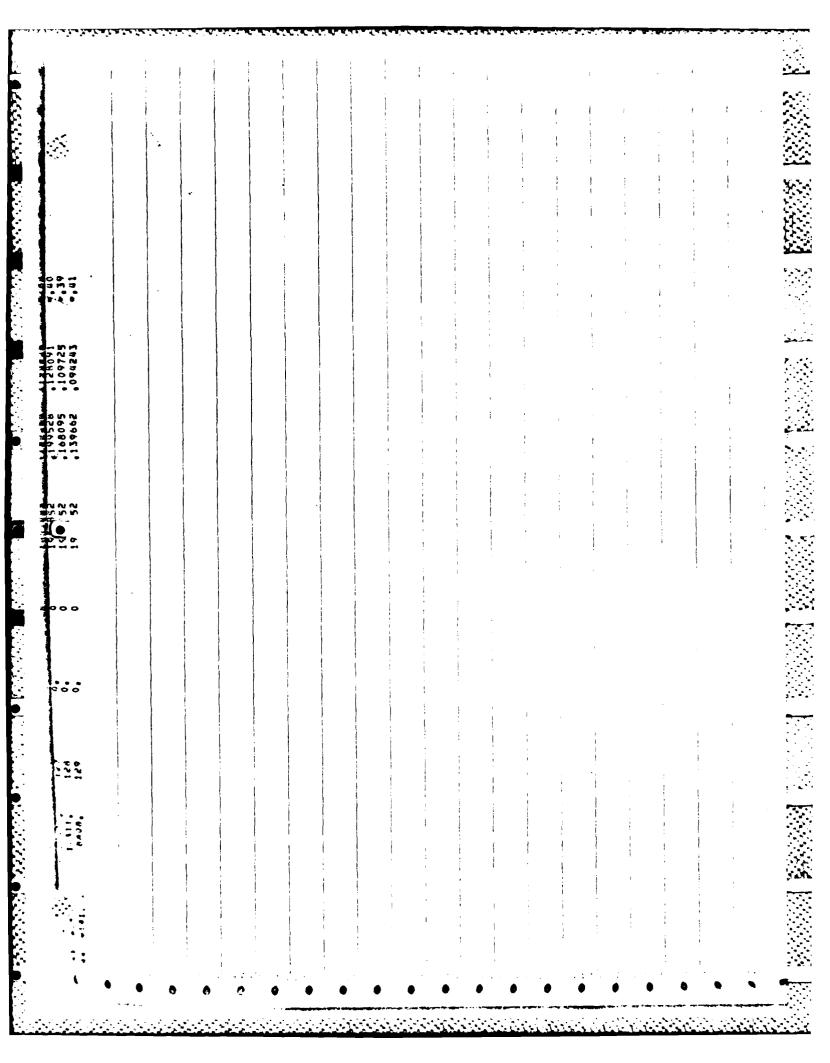
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MAVE EQUATION ANALYSIS FOR 42-IN, DIAMETER PIPE PILES MC CLELLAND SUIL REPURT DATA FOR ACMR 3-PILE STRUCTURES == BORING SITES 3A + 4 8 JUNE 1976

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Pile Driving Resistance Curves - Insert Piling

 Pile Diameter
 - 33 in.

 Minimum Wall Thickness
 -1.00 in.

 Penetration
 - 75 ft.

 - 150 ft.

 Hammer
 - Vulcan 040

 Quake Factor, Tip
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 - .10 in.

 - .30 in.

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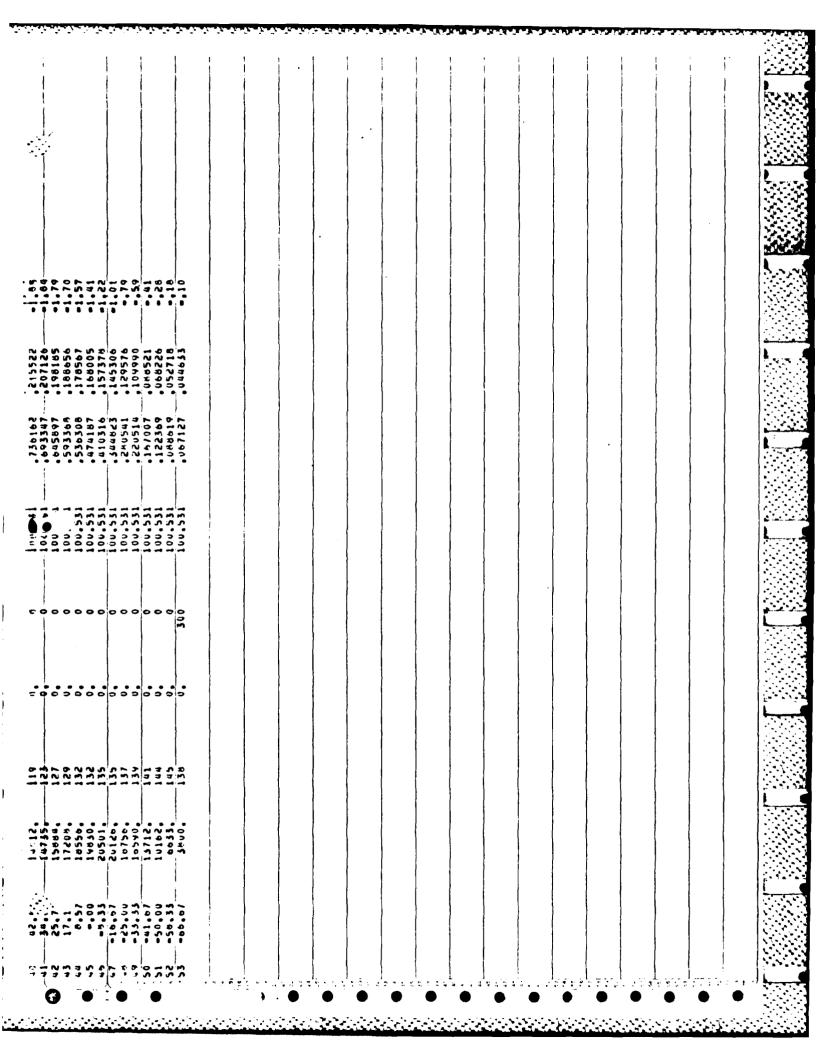
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MAVE ELUATION AVALYSIS FOR 33-IN. DIAMETER PIPE PILES MC CLELLAND SUIL REPURT DATA FUR ACHN 3-PILE STRUCTUMES -- U.S.NAVY 25 august 1470

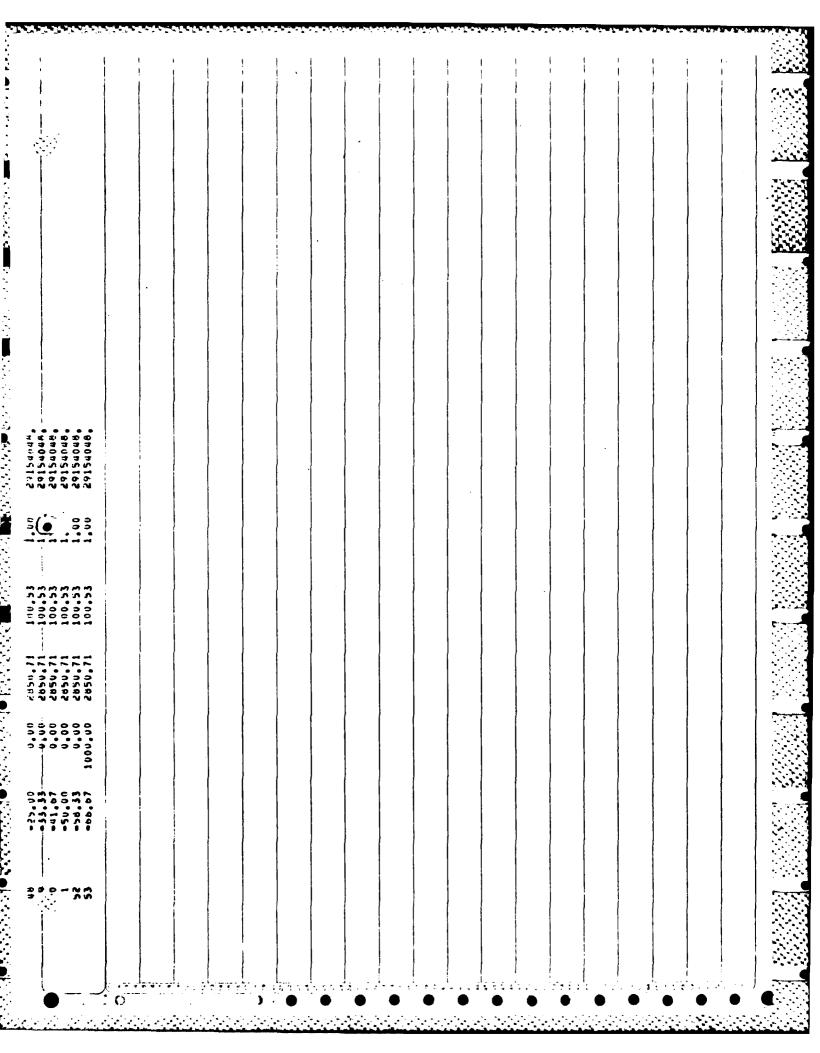
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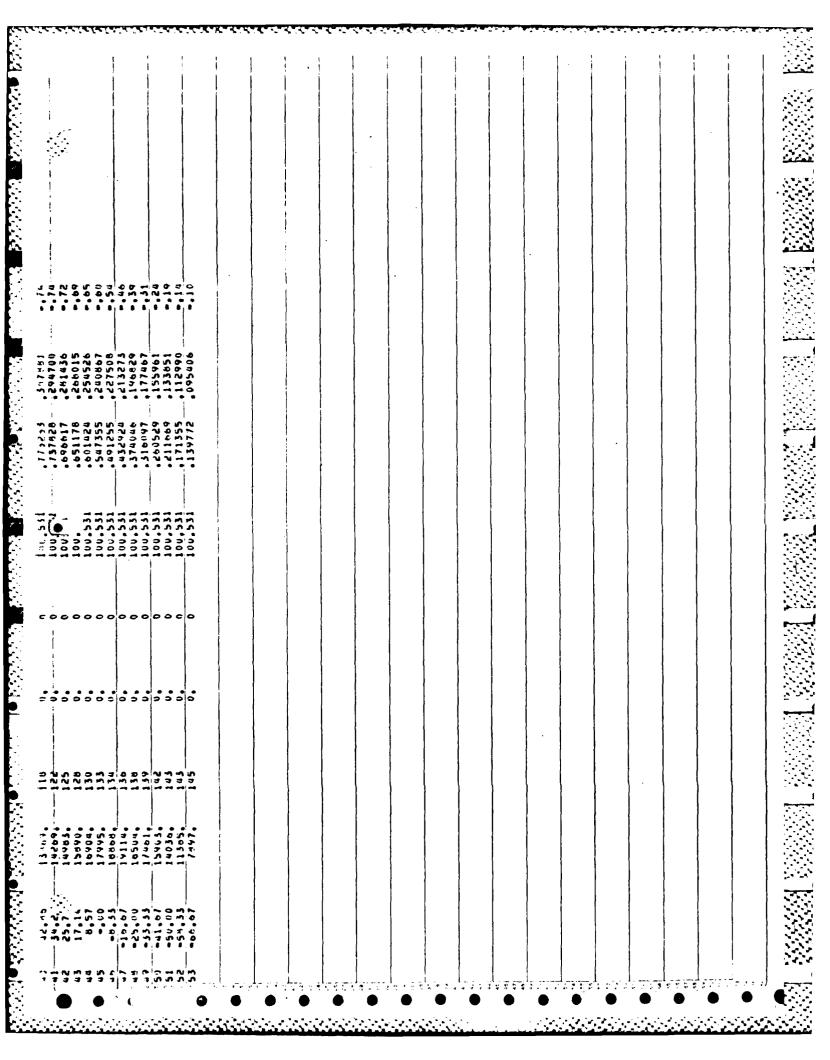
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MAVE EQUATION ANALYSIS FUR 33-IN. DIAMETER PIPE PILES MC CLELLAND SUIL REPURI DATA FUR ACMM 3-PILE SINUCTUMES U.S.NAVY	
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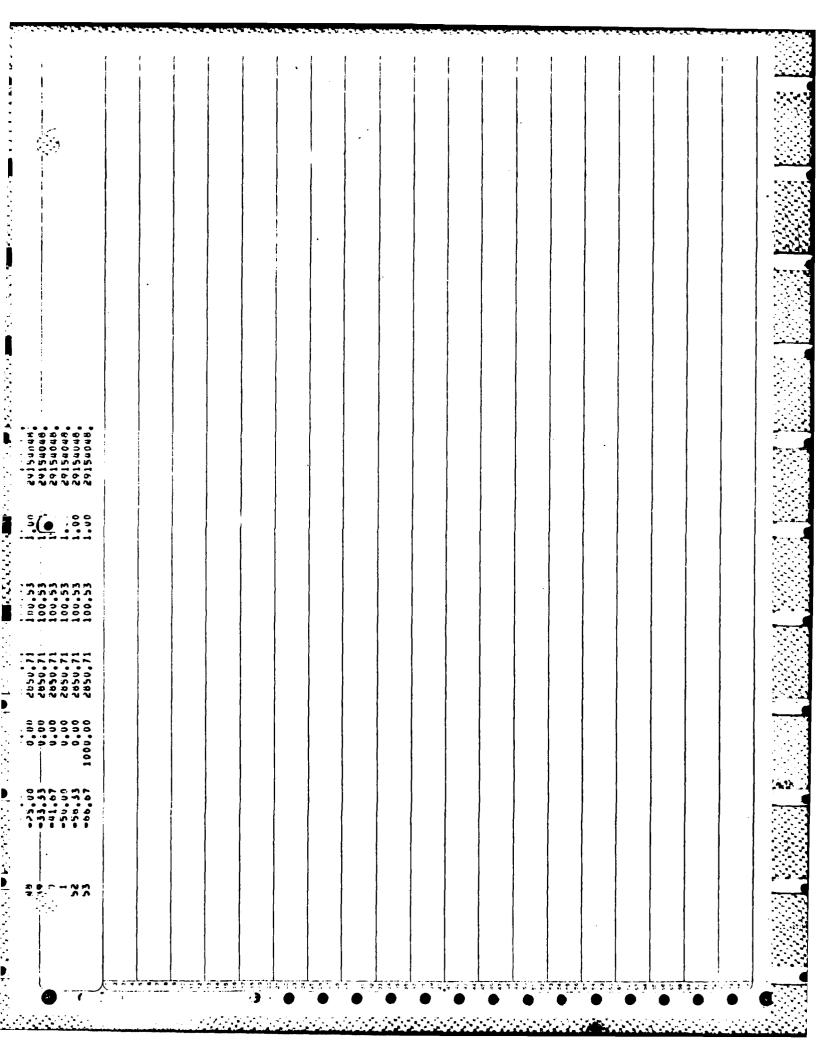
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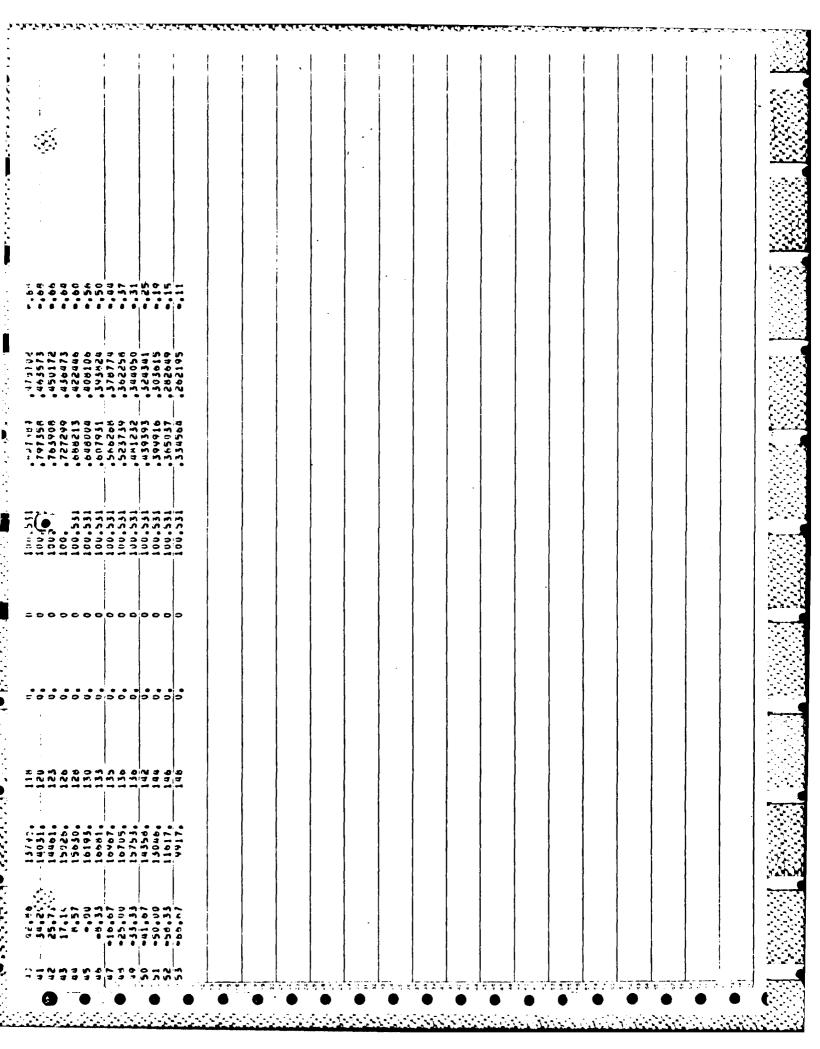
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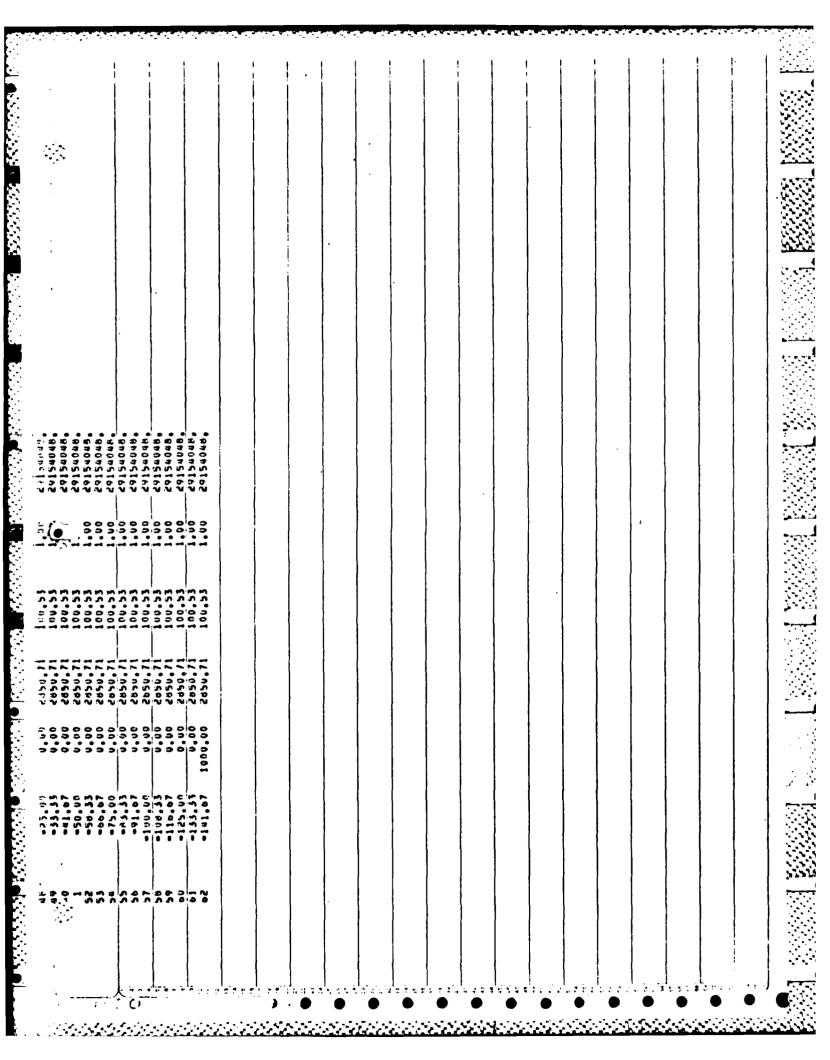
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STATION NUMBER TUP HUTIOM 446666666 LENGTH (FT) 2000 CONTRACT SELECTIONS AND CONTRACTOR SELECTIONS OF PARE BIANDING PILE(FT) --- 560,000 NUMBER OF THE RESISTANCE PERCENTAGES
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SOIL CURAC FOR SIVE - GPUINT TABLE 6 -- SPECIFIED BLOW CUUNT DATA MALL THICKNESS (IN) NUMBER OF SPECIFIED BLUM COUNTS TULEHANCE 25/25 MATERIAL IYPE TIP MESISTANCE 14.0000 TABLE S -- SUIL UATA BLUFS FER 150. 250. 300. SECTION

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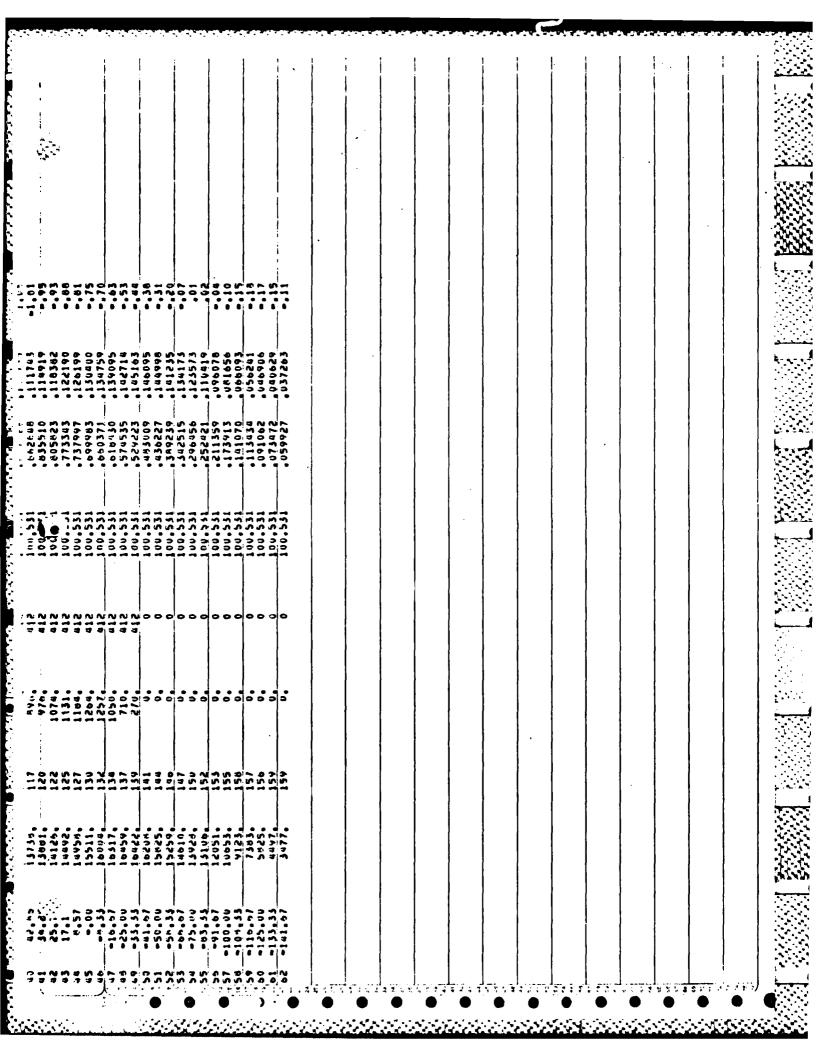


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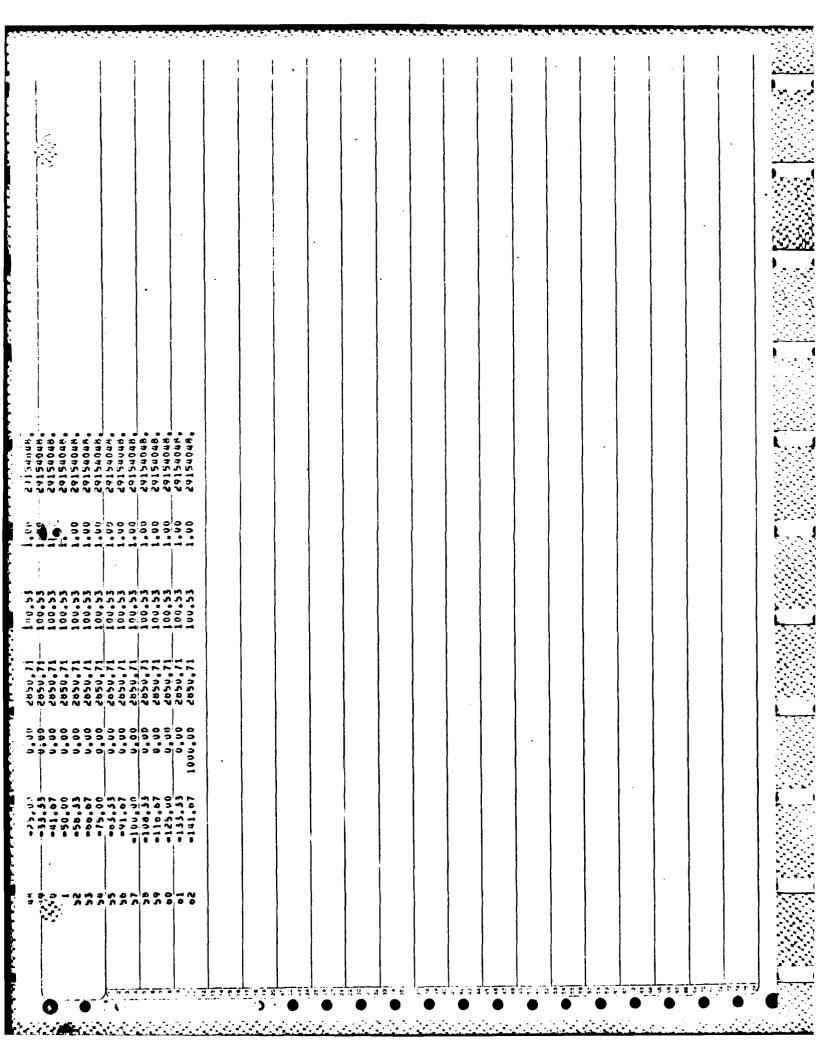
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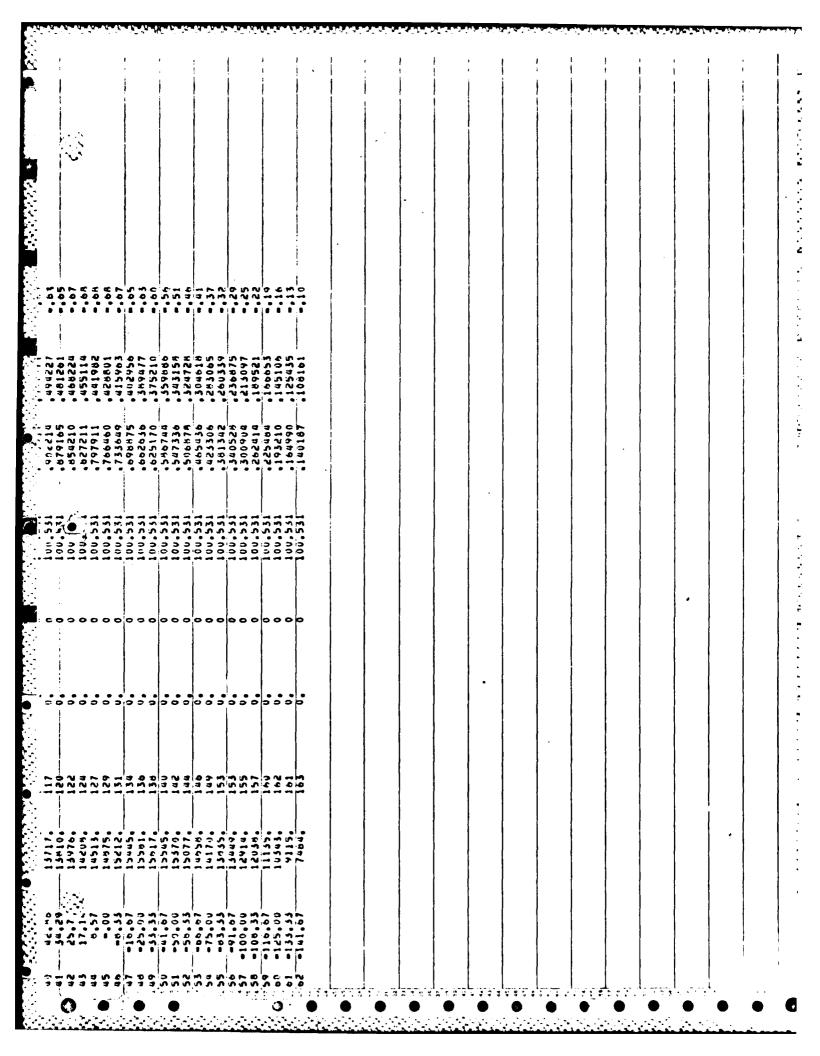
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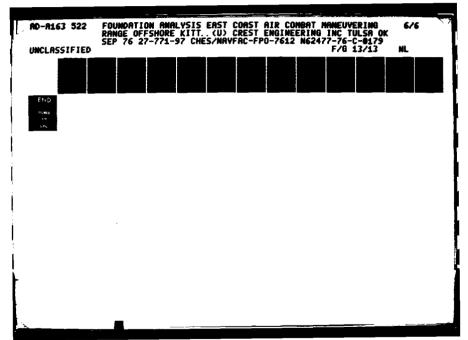
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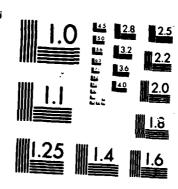
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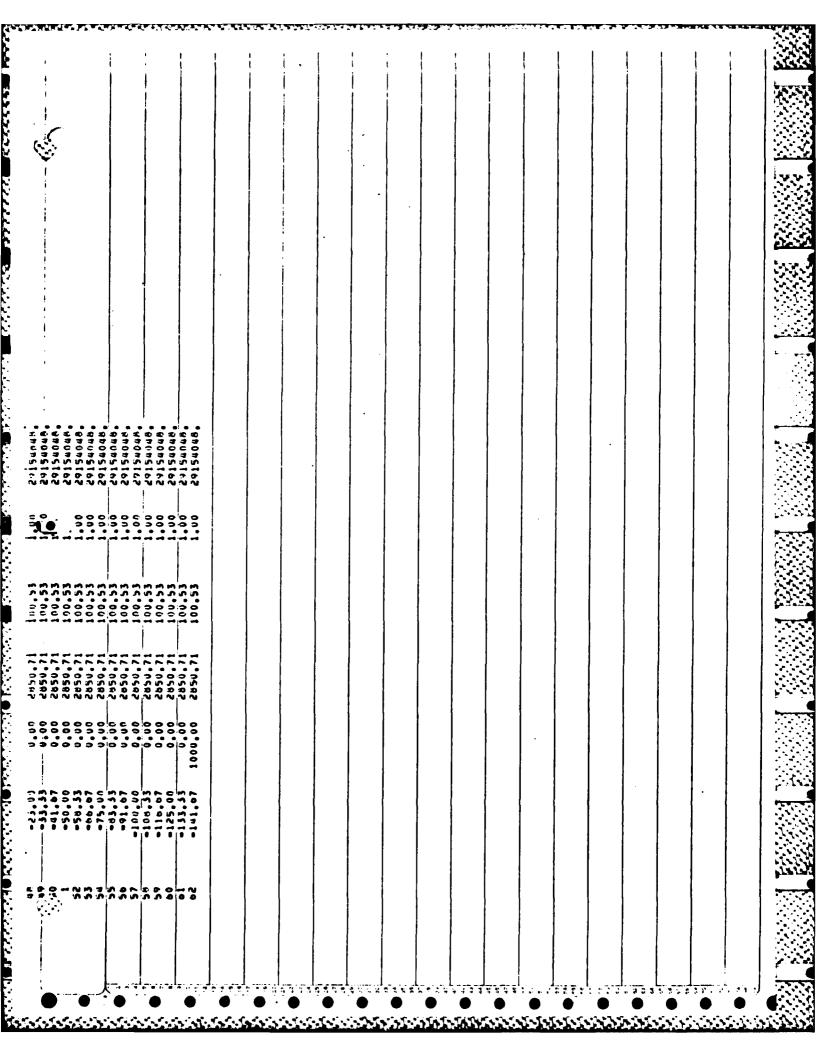
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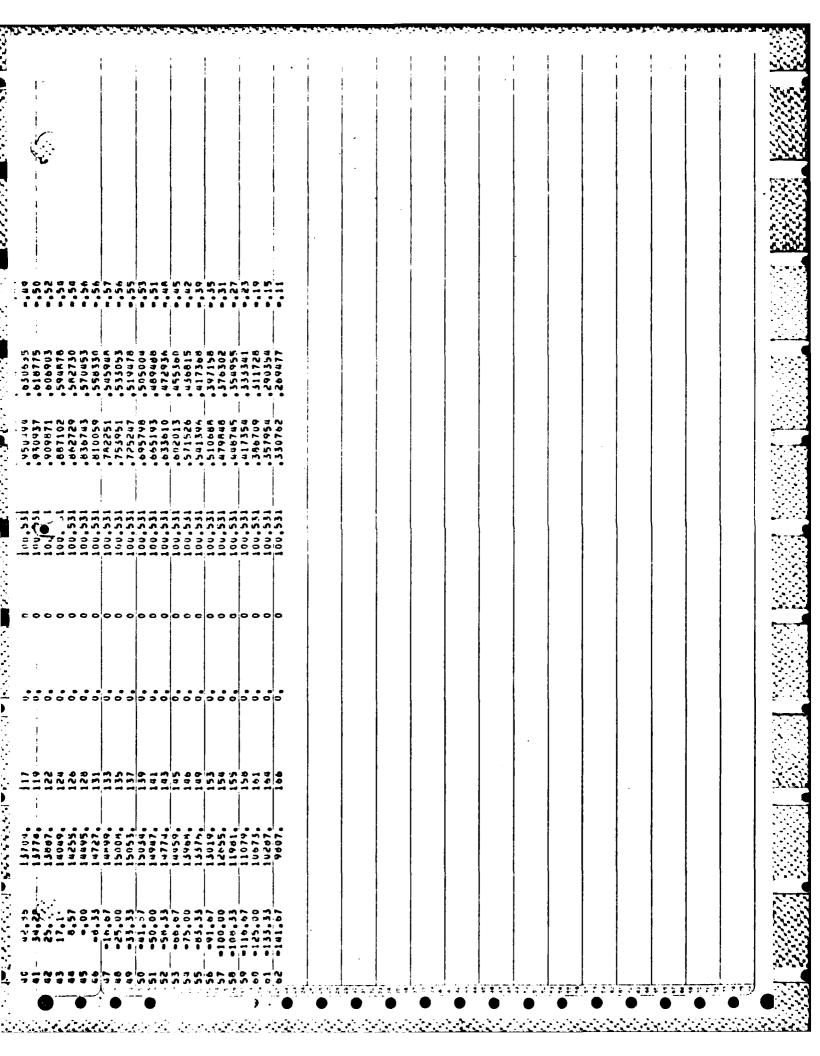
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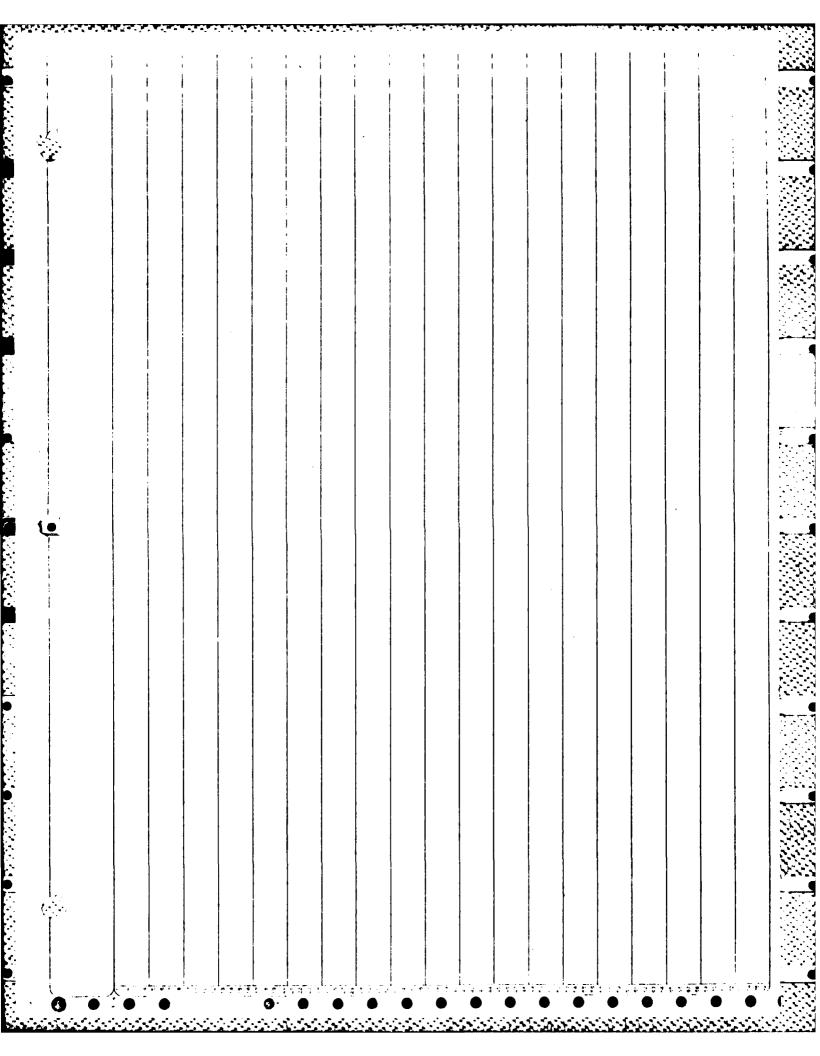


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